Attachment A10

Pedestrian Wind Environment Assessment 757-763 George Street, Haymarket





PEDESTRIAN WIND ENVIRONMENT STUDY

757-763 GEORGE STREET, HAYMARKET

WD154-07F01(REV5)- WE REPORT

OCTOBER 30, 2020

Prepared for:

Samprian Pty Ltd

447-451 Parramatta Road, Leichardt NSW 2040

 WINDTECH Consultants Pty Ltd

 Head Office: 607 Forest Road, Bexley, NSW 2207, Australia

 P +61 2 9503 0300
 E reception@windtechglobal.com

 W www.windtechconsult.com

DOCUMENT CONTROL

Date	Revision History	Issued Revision	Prepared By (initials)	Instructed By (initials)	Reviewed & Authorised by (initials)
October 2, 2020	Initial.	0	NR	SR	JG
October 7, 2020	Revised Base Case results	1	NR	SR	AB
October 12, 2020	Revised Discussion	2	NR	SR	TR
October 19, 2020	Revised Results Table for Base Case	3	NR	SR	JG
October 28, 2020	Revised Base Case results	4	NR	SR	JG
October 30, 2020	Additional figures and comments	5	NR	SR	JG

The work presented in this document was carried out in accordance with the Windtech Consultants Quality Assurance System, which is based on International Standard ISO 9001.

This document is issued subject to review and authorisation by the Team Leader noted by the initials printed in the last column above. If no initials appear, this document shall be considered as preliminary or draft only and no reliance shall be placed upon it other than for information to be verified later.

This document is prepared for our Client's particular requirements which are based on a specific brief with limitations as agreed to with the Client. It is not intended for and should not be relied upon by a third party and no responsibility is undertaken to any third party without prior consent provided by Windtech Consultants. The information herein should not be reproduced, presented or reviewed except in full. Prior to passing on to a third party, the Client is to fully inform the third party of the specific brief and limitations associated with the commission.

EXECUTIVE SUMMARY

This report presents the results of a detailed investigation into the wind environment impact of the development located at 757-763 George Street, Haymarket. Testing was performed at Windtech's boundary layer wind tunnel facility. The wind tunnel has a 3.0m wide working section and a fetch length of 14m, and measurements were taken from 16 wind directions at 22.5 degree increments. Testing was carried out using a 1:300 detailed scale model of the development. The effects of nearby buildings and land topography have been accounted for through the use of a proximity model which represents an area with a radius of 375m.

Testing was performed for five massing variations of the development, as well as for the existing site conditions, which are denoted by the following scenarios:

- With the existing surrounding buildings and the inclusion of the Base Case Massing. In this report, this test case is referred to as the "Base Case Massing".
- With the existing surrounding buildings and the inclusion of Proposed Case Massing. In this report, this test case is referred to as the "Proposed DCP Envelope".
- With the existing surrounding buildings and the inclusion of Envelope 1 Massing. In this report, this test case is referred to as the "Alternative Envelope A".
- With the existing surrounding buildings and the inclusion of Envelope 2 Massing. In this report, this test case is referred to as the "Alternative Envelope B".
- With the existing surrounding buildings and the inclusion of Envelope 3 Massing. In this report, this test case is referred to as the "Alternative Envelope C".
- With the existing surrounding buildings and the existing building on the subject development site. In this report, this test case is referred to as the "Existing Site".

Peak gust and mean wind speeds were measured at selected critical outdoor trafficable locations within and around the subject development. Wind velocity coefficients representing the local wind speeds are derived from the wind tunnel and are combined with a statistical model of the regional wind climate (which accounts for the directional strength and frequency of occurrence of the prevailing regional winds) to provide the equivalent full-scale wind speeds at the site. The wind speed measurements are compared with criteria for pedestrian comfort and safety, based on Gust-Equivalent Mean (GEM) and annual maximum gust winds, respectively.

The model was tested in the wind tunnel without the effect of any forms of wind ameliorating devices. The results of the study indicate that wind conditions for the majority of trafficable outdoor locations within and around the development will be suitable for their intended uses. The effect of vegetation was also excluded from the testing. In-principle recommendations are suggested as follows:

- an impermeable awning along the northern and eastern building aspects with a small return along the south aspect.
- It is also recommended that the level of the podium that meets the base of the tower be configured at the north-western corner to direct winds around that corner and towards the south-west and to minimise downwash of the north-east winds off the northern aspect being directed westward. The necessary configuration at the base of the tower at its north-western corner could potentially relate to the inclusion of an increased setback or cut out at the affected location and can be accommodated within the extent of the DCP Envelope at a later stage.

With the inclusion of the above treatments in the final design, it is expected that wind conditions for all outdoor trafficable areas within and around the Proposed Envelopes will be equivalent to or better than the wind conditions pertaining to the Base Case Massing.

CONTENTS

1	Intro	duction	1
2	Wind	Tunnel Model	2
3	Boun	dary Layer Wind Profiles at the Site	14
4	Regio	nal Wind Model	17
5	Pede	strian Wind Comfort and Safety	20
	5.1	Measured Wind Speeds	20
	5.2	Wind Speed Criteria Used for This Study	20
	5.3	Layout of Study Points	22
6	Resu	ts and Discussion	24
7	Refer	ences	37

Appendix A	Published Environmental Criteria
Appendix B	Data Acquisition
Appendix C	Directional Plots of Wind Tunnel Results
Appendix D	Velocity and Turbulence Intensity Profiles

1 INTRODUCTION

A wind tunnel study has been undertaken to assess wind speeds at selected critical outdoor trafficable areas within and around the subject development. The test procedures followed for this wind tunnel study were based on the guidelines set out in the Australasian Wind Engineering Society Quality Assurance Manual (AWES-QAM-1-2019), ASCE 7-16 (Chapter C31), and CTBUH (2013).

A scale model of the development was prepared, including the surrounding buildings and land topography. Testing was performed at Windtech's boundary layer wind tunnel facility. The wind tunnel has a 3.0m wide working section and a fetch length of 14m, and measurements were taken from 16 wind directions at 22.5 degree increments. The wind tunnel was configured to the appropriate boundary layer wind profile for each wind direction. Wind speeds were measured using Dantec hot-wire probe anemometers, positioned to monitor wind conditions at critical outdoor trafficable areas of the development.

The model was tested in the wind tunnel without the effect of any forms of wind ameliorating devices such as screens, balustrades, etc., which are not already shown in the architectural drawings. The effect of vegetation was also excluded from the testing. The wind speeds measured during testing were combined with a statistical model of the regional wind climate to provide the equivalent full-scale wind speeds at the site. The measured wind speeds were compared against appropriate criteria for pedestrian comfort and safety, and in-principle treatments have been recommended for any area which was exposed to strong winds. These treatments could be in the form of retaining vegetation that is already proposed for the site, or including additional vegetation, screens, awnings, etc. Note however that, in accordance with the AWES Guidelines (2014), only architectural elements or modifications are used to treat winds which represent an exceedance of the existing wind conditions and exceed the safety limit.

2 WIND TUNNEL MODEL

Wind tunnel testing was carried out using a 1:300 scale model of the development and surroundings. The study model incorporates all necessary architectural features on the façade of the development to ensure an accurate wind flow is achieved around the model, and was constructed using a Computer Aided Manufacturing (CAM) process to ensure that a high level of detail and accuracy is achieved. The effect of nearby buildings and land topography has been accounted for through the use of a proximity model, which represents a radius of 375m from the development site. Photographs of the wind tunnel model are presented in Figures 1 . A plan of the proximity model is provided in Figure 2.

Testing was performed for five massing variations of the development, as well as for the existing site conditions, which are denoted by the following scenarios:

- With the existing surrounding buildings and the inclusion of the Base Case Massing. In this report, this test case is referred to as the "Base Case Massing".
- With the existing surrounding buildings and the inclusion of Proposed Case Massing. In this report, this test case is referred to as the "Proposed DCP Envelope".
- With the existing surrounding buildings and the inclusion of Envelope 1 Massing. In this report, this test case is referred to as the "Alternative Envelope A".
- With the existing surrounding buildings and the inclusion of Envelope 2 Massing. In this report, this test case is referred to as the "Alternative Envelope B".
- With the existing surrounding buildings and the inclusion of Envelope 3 Massing. In this report, this test case is referred to as the "Alternative Envelope C".
- With the existing surrounding buildings and the existing building on the subject development site. In this report, this test case is referred to as the "Existing Site".



Figure 1a: Photograph of the Wind Tunnel Model (Proposed DCP Envelope, view from the south)



Figure 1b: Photograph of the Wind Tunnel Model (Proposed DCP Envelope, view from the east)



Figure 1c: Close up photograph of the Wind Tunnel Model (Proposed DCP Envelope, view from the north)



Figure 1d: Close up photograph of the Wind Tunnel Model (Proposed DCP Envelope, view from the west)

Setbacks

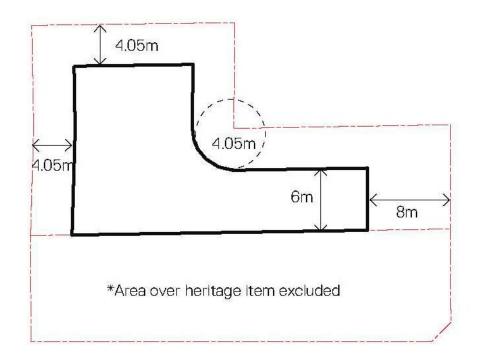


Figure 1e: Base Case Tower Architectural Plan (Appendix E – 757-763 George Street Draft Urban Design Report)

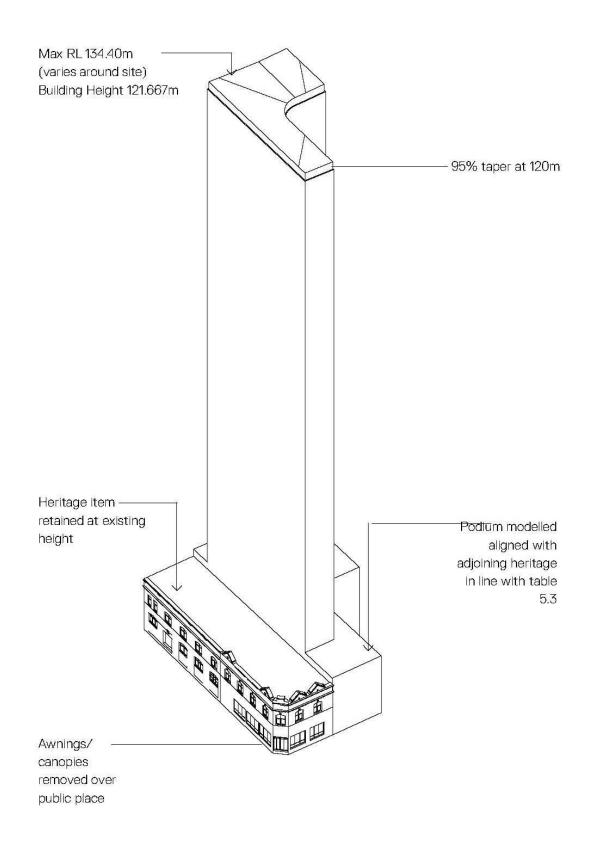






Figure 1g: Photograph of the Wind Tunnel Model (Base Case Massing, view from the south-east)



Figure 1h: Photograph of the Wind Tunnel Model (Proposed DCP Envelope, view from the south-east)



Figure 1i: Photograph of the Wind Tunnel Model (Alternative Envelope A, view from the south-east)



Figure 1j: Photograph of the Wind Tunnel Model (Alternative Envelope B, view from the south-east)



Figure 1k: Photograph of the Wind Tunnel Model (Alternative Envelope C, view from the south-east)



Figure 11: Photograph of the Wind Tunnel Model (Existing Site, view from the south-east)



Figure 2: Proximity Model Plan

3 BOUNDARY LAYER WIND PROFILES AT THE SITE

The roughness of the surface of the earth has the effect of slowing down the wind near the ground. This effect is observed up to the boundary layer height, which can range between 500m to 3km above the earth's surface depending on the roughness of the surface (ie: oceans, open farmland, etc). Within this range the prevailing wind forms a boundary layer wind profile.

Various wind codes and standards and other publications classify various types of boundary layer wind flows depending on the surface roughness z_0 . Descriptions of typical boundary layer wind profiles, based on Deaves & Harris (1978), are summarised as follows:

- Flat terrain ($0.002m < z_0 < 0.003m$). Examples include inland water bodies such as lakes, dams, rivers, etc, and the open ocean.
- Semi-open terrain (0.006m < z_0 < 0.01m). Examples include flat deserts and plains.
- Open terrain ($0.02m < z_0 < 0.03m$). Examples include grassy fields, semi-flat plains, and open farmland (without buildings or trees).
- Semi-suburban/semi-forest terrain ($0.06m < z_0 < 0.1m$). Examples include farmland with scattered trees and buildings and very low-density suburban areas.
- Suburban/forest terrain ($0.2m < z_0 < 0.3m$). Examples include suburban areas of towns and areas with dense vegetation such as forests, bushland, etc.
- Semi-urban terrain (0.6m < z_0 < 1.0m). Examples include centres of small cities, industrial parks, etc.
- Urban terrain (2.0m < z_0 < 3.0m). Examples include centres of large cities with many high-rise towers, and also areas with many closely-spaced mid-rise buildings.

The boundary layer wind profile does not change instantly due to changes in the terrain roughness. It can take many kilometres (at least 100km) of a constant surface roughness for the boundary layer wind profile to achieve a state of equilibrium. Hence an analysis of the effect of changes in the upwind terrain roughness is necessary to determine an accurate boundary layer wind profile at the development site location.

For this study this has been undertaken based on the method given in AS/NZS1170.2:2011, which uses a "fetch" length of 60 times the study reference height. However, it should be noted that this "fetch" commences *beyond* a "lag distance" area, which has a length of 20 times the study reference height (in accordance with AS/NZS1170.2:2011), so the actual "fetch" of terrain analysed is the area between 20 and 60 times the study reference height away from the site. The proximity model accounts for the effect of the near field topographic effects as well as the influence of the local built forms.

An aerial image showing the surrounding terrain is presented in Figure 3 for a range of 3.6km from the edge of the proximity model used for the wind tunnel study. The resulting mean and gust terrain and height multipliers at the site location are presented in Table 1, referenced to the study reference height (which is approximately half of the height of the Base Case Massing, the tallest of the massing cases, since typically we are most interested in the wind effects at the ground plane). Details of the boundary layer wind profiles at the site are combined with the regional wind model (see Section 4) to determine the site wind speeds.

	Terrain and Height Multiplier			Turbulence	Equivalent Terrain	
Wind Sector (degrees)	k _{tr,T=1hr} (hourly)	k _{tr,T=10min} (10min)	k _{tr,T=3s} (3sec)	Intensity I _v	Category (AS/NZS1170.2:2011 naming convention)	
0	0.77	0.80	1.18	0.180	2.7	
30	0.83	0.86	1.21	0.157	2.3	
60	0.75	0.78	1.17	0.189	2.8	
90	0.71	0.75	1.14	0.204	3.0	
120	0.79	0.83	1.19	0.169	2.5	
150	0.75	0.79	1.17	0.187	2.8	
180	0.61	0.65	1.08	0.256	3.5	
210	0.71	0.75	1.15	0.202	3.0	
240	0.71	0.75	1.15	0.202	3.0	
270	0.71	0.75	1.15	0.202	3.0	
300	0.76	0.80	1.17	0.182	2.7	
330	0.78	0.82	1.18	0.175	2.6	

Table 1: Approaching Boundary Layer Wind Profile Analysis Summary(at the study reference height)

For each of the 16 wind directions tested in this study, the approaching boundary layer wind profiles modelled in the wind tunnel closely matched the profiles listed in Table 1. Plots of the boundary layer wind profiles used for the wind tunnel testing are presented in Appendix D of this report.

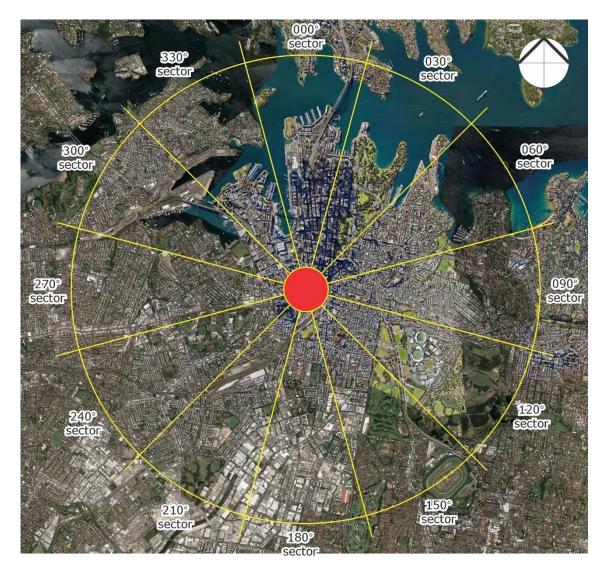


Figure 3: Aerial Image of the Surrounding Terrain (radius of 3.6km from the edge of the proximity model, which is coloured red)

4 REGIONAL WIND MODEL

The regional wind model used in this study was determined from an analysis of measured directional mean wind speeds obtained at the meteorological recording station located at Kingsford Smith Airport (Sydney Airport). Data was collected from 1995 to 2016 between 6am to 10pm and corrected so that it represents wind speeds over standard open terrain at a height of 10m above ground for each wind direction. From this analysis, directional probabilities of exceedance and directional wind speeds for the region are determined. The directional wind speeds are summarised in Table 2. The directional wind speeds and corresponding directional frequencies of occurrence are presented in Figure 4.

The data indicates that the southerly winds are by far the most frequent winds for the Sydney region, and are also the strongest. The westerly winds occur most frequently during the winter season for the Sydney region, and although they are typically not as strong as the southerly winds, they are usually a cold wind and hence can be a cause for discomfort for outdoor areas. North-easterly winds occur most frequently occur during the warmer months of the year for the Sydney region, and hence are usually welcomed within outdoor areas since they are typically not as strong as the southerly winds as strong as the southerly winds.

The recurrence intervals examined in this study are for exceedances of 5% (per 90 degree sector) for the pedestrian comfort criteria using Gust-Equivalent Mean (GEM) wind speeds, and annual maximum wind speeds (per 22.5 degree sector) for the pedestrian safety criterion. Note that the 5% probability wind speeds presented in Table 2 are only used for the directional plot presented in Figure 4 and are not used for the integration of the probabilities.

Wind Direction	5% Exceedance	Annual Maximum
N	5.9	9.9
NNE	9.9	12.9
NE	9.7	12.3
ENE	7.5	10.0
E	6.3	9.3
ESE	6.2	9.1
SE	7.0	10.1
SSE	8.5	12.2
S	10.3	13.9
SSW	10.0	14.1
SW	6.9	11.9
WSW	9.3	13.6
W	9.8	14.4
WNW	8.8	14.3
NW	6.7	12.6
NNW	5.5	10.7

Table 2: Directional Wind Speeds (m/s)(hourly means, referenced to 10m above ground in standard open terrain)

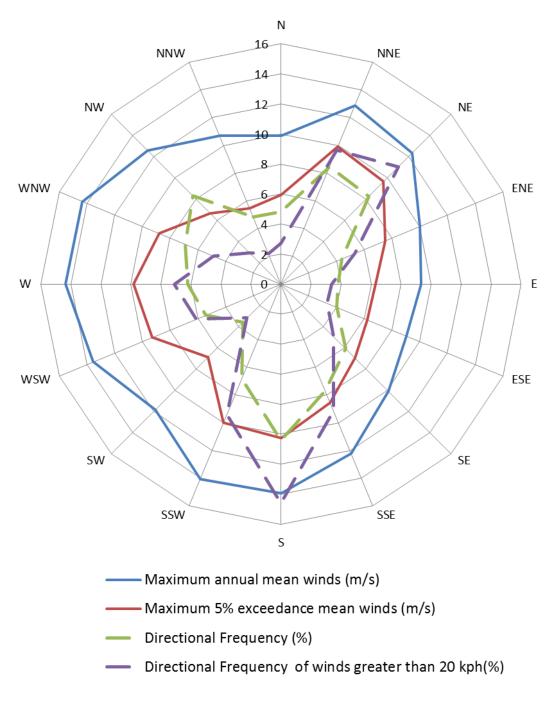


Figure 4: Annual and 5% Exceedance Hourly Mean Wind Speeds, and Frequencies of Occurrence, for the Sydney Region (referenced to 10m above ground in standard open terrain)

5 PEDESTRIAN WIND COMFORT AND SAFETY

The acceptability of wind conditions of an area is determined by comparing the measured wind speeds against an appropriate criteria. This section outlines how the measured wind speeds were obtained, the criteria considered for the development, as well as the critical trafficable areas that were assessed and their corresponding criteria designation.

5.1 Measured Wind Speeds

Wind speeds were measured using Dantec hot-wire probe anemometers, positioned to monitor wind conditions at critical outdoor trafficable areas of the development. The reference mean free-stream wind speed measured in the wind tunnel, which is at a full-scale height of 200m and measured 3m upstream of the study model.

Measurements were acquired for 16 wind directions at 22.5 degree increments using a sample rate of 1,024Hz. The full methodology of determining the wind speed measurements at the site from the Dantec Hot-wire probe anemometers is provided in Appendix B. Based on the results of the analysis of the boundary layer wind profiles at the site (see Section 3), and incorporating the regional wind model (see Section 4), the data sampling length of the wind tunnel test for each wind direction corresponds to a full-scale sample length ranging between 30 minutes and 1 hour. Research by A.W. Rofail and K.C.S. Kwok (1991) has shown that, in addition to the mean and standard deviation of the wind being stable for sample lengths of 15 minutes or more (full-scale), the peak value determined using the upcrossing method is stable for sample lengths of 30 minutes or more.

5.2 Wind Speed Criteria Used for This Study

For this study, the measured wind conditions for the various critical outdoor trafficable areas around the subject development are compared against the criteria presented in Section 5.1.9 of the Draft Sydney Development Control Plan 2012 - Central Sydney Planning Review Amendment, which supersedes the criteria detailed in the City of Sydney Development Control Plan 2012 (SDCP2012).

For pedestrian comfort, the Draft Sydney DCP 2012 requires that the hourly mean wind speed, or Gust-Equivalent Mean (GEM) wind speed (whichever is greater for each wind direction), must not exceed 8m/s for walking, 6m/s for standing, and 4m/s for sitting. These are based on a 5% probability of exceedance.

For pedestrian safety, the Draft Sydney DCP 2012 defines a safety limit criterion of 24m/s, based on an annual maximum 0.5 second gust wind speed, which applies to all areas.

Furthermore, in accordance with the provisions of the Draft Sydney DCP 2012, the existing conditions for the pedestrian footpaths around the site are also analysed as part of this study to determine the impact of the subject development. If it is found that the existing conditions

exceed the relevant criteria, then the target wind speed for that area with the inclusion of the proposed development is to at least match the existing site conditions.

In accordance with the provisions of the Draft Sydney DCP 2012, the wind speed assessment is undertaken for winds occurring between 6am and 10pm (AEST). A more detailed comparison of published criteria for pedestrian wind comfort and safety is provided in Appendix A.

For this study the measured wind conditions of the selected critical outdoor trafficable areas are compared against two sets of criteria; one for pedestrian safety, and one for pedestrian comfort. The safety criterion is applied to the annual maximum gust winds, and the comfort criteria is applied to Gust Equivalent Mean (GEM) winds. In accordance with ASCE (2003), the GEM wind speed is defined as follows:

$$GEM = max\left(\bar{V}, \frac{\hat{V}}{1.85}\right) \tag{5.1}$$

Where:

 ${ar V}$ is the mean wind speed.

 \widehat{V} is the gust wind speed.

The criteria considered in this study are summarised in Tables 3 and 4 for pedestrian comfort and safety, respectively. The results of the wind tunnel study are presented in the form of directional plots attached in Appendix C of this report. For each study point there is a plot of the GEM wind speeds using the comfort criteria, and a plot for the annual maximum gust wind speeds using the safety criterion.

Classification	Description	Maximum 5% Exceedance GEM Wind Speed (m/s)
Sitting	Outdoor areas that involve seating such as parks, dining areas in restaurants, amphitheatres, etc.	4
Standing	Short duration stationary activities (generally less than 1 hour), including window shopping, waiting areas, etc.	6
Walking	For pedestrian thoroughfares, private swimming pools, most communal areas, private balconies and terraces, etc.	8

Table 3: Pedestrian Comfort Criteria (Draft Sydney DCP 2012)

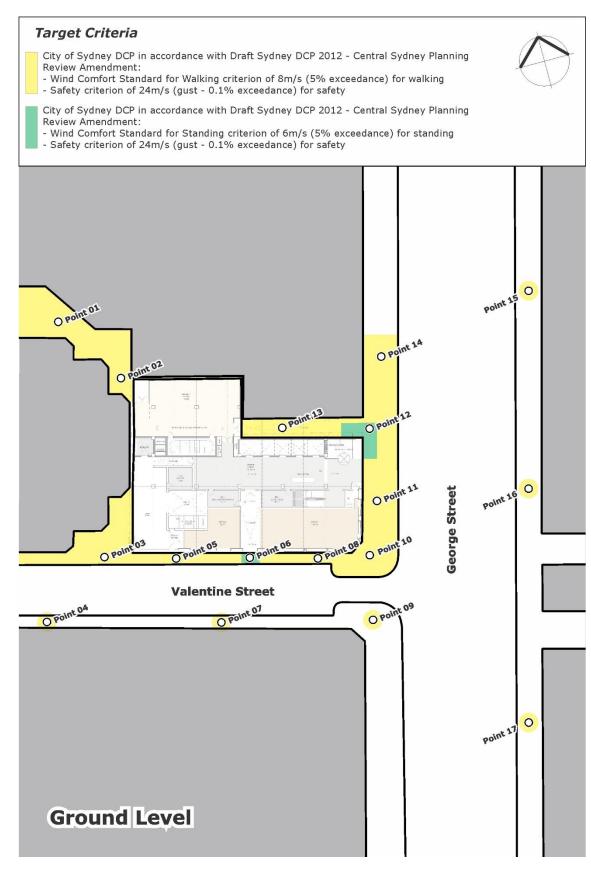
Table 4: Pedestrian Safety Criterion (Draft Sydney DCP 2012)

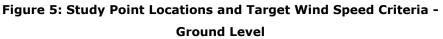
Classification	Description	Annual Maximum Gust Wind Speed (m/s)
Safety	Safety criterion applies to all trafficable areas.	24

5.3 Layout of Study Points

For this study, a total of 17 study point locations on the Ground Level along the pedestrian footpaths along George Street and Valentine Street around the proposed development site were selected for analysis in the wind tunnel.

The locations of the various study points tested for this study, as well as the target wind speed criteria for the various outdoor trafficable areas of the development, are presented in Figure 5 in the form of a marked-up plan. It should be noted that only the most critical outdoor locations of the development have been selected for analysis.





6 **RESULTS AND DISCUSSION**

The results of the wind tunnel study are presented in the form of directional plots in Appendix C for all study points locations, summarised in Tables 5, and shown on marked-up plans in Figures 6 for each of the scenarios. The wind speed criteria that the wind conditions should achieve are also listed in Tables 5 for each study point location, as well as in Figure 5.

The results of the study indicate that wind conditions for the majority of trafficable outdoor locations within and around the development will be suitable for their intended uses. The effect of vegetation was also excluded from the testing. In-principle recommendations are suggested as follows:

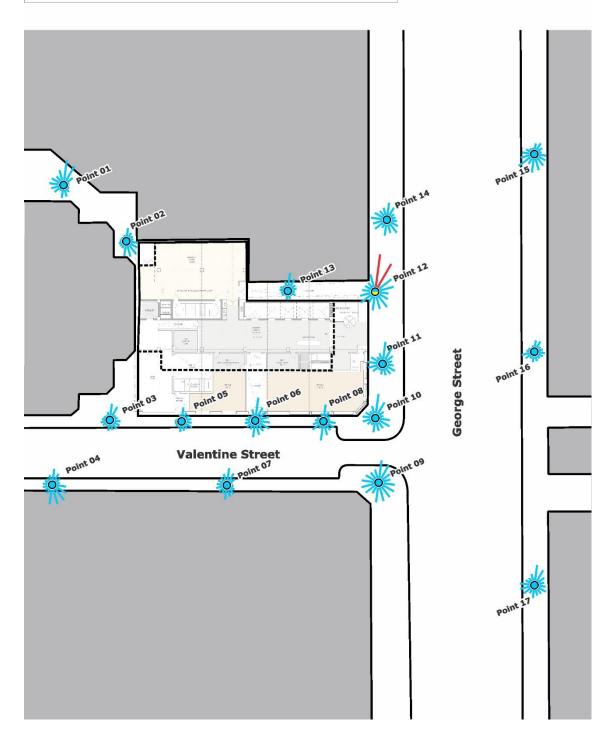
- An impermeable awning along the northern and eastern building aspects with a small return along the south aspect. See Figure 7.
- It is also recommended that the level of the podium that meets the base of the tower be configured at the north-western corner to direct winds around that corner and towards the south-west and to minimise downwash of the north-east winds off the northern aspect being directed westward. The necessary configuration at the base of the tower at its north-western corner could potentially relate to the inclusion of an increased setback or cut out at the affected location and can be accommodated within the extent of the DCP Envelope at a later stage.

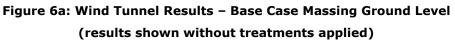
With the inclusion of the above treatments in the final design, it is expected that wind conditions for all outdoor trafficable areas within and around the Proposed Envelopes will be equivalent to or better than the wind conditions pertaining to the Base Case Massing.

As a general note, the use of loose glass-tops and light-weight sheets or covers (including loose BBQ lids) is not appropriate on high-rise outdoor terraces and balconies. Furthermore, lightweight furniture is not recommended unless it is securely attached to the balcony or terrace floor slab.



- Wind Speed Magnitude from Directions Satisfying Criteria
- Passing Safety Limit and Comfort Criteria
- 🔴 Failing Safety Limit
- 🔵 Failing Comfort Criteria
- Failing Safety Limit and Comfort Criteria







- Wind Speed Magnitude from Directions Satisfying Criteria
- Passing Safety Limit and Comfort Criteria
- 🔴 Failing Safety Limit
- 🔵 Failing Comfort Criteria
- Failing Safety Limit and Comfort Criteria

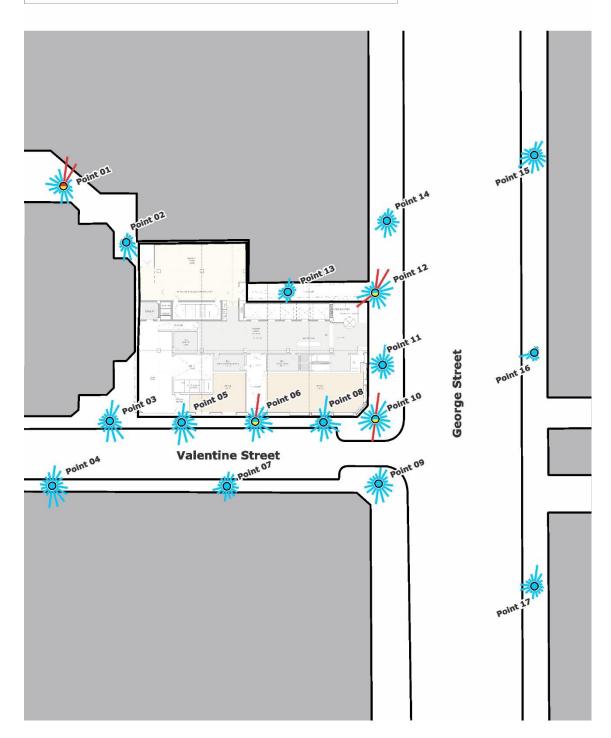


Figure 6b: Wind Tunnel Results – Proposed DCP Envelope Ground Level (results shown without treatments applied)

Wind Speed Magnitude from Directions Exceeding Criteria

- Wind Speed Magnitude from Directions Satisfying Criteria
- Passing Safety Limit and Comfort Criteria
- 🔵 🛛 Failing Safety Limit
- Failing Comfort Criteria
- Failing Safety Limit and Comfort Criteria

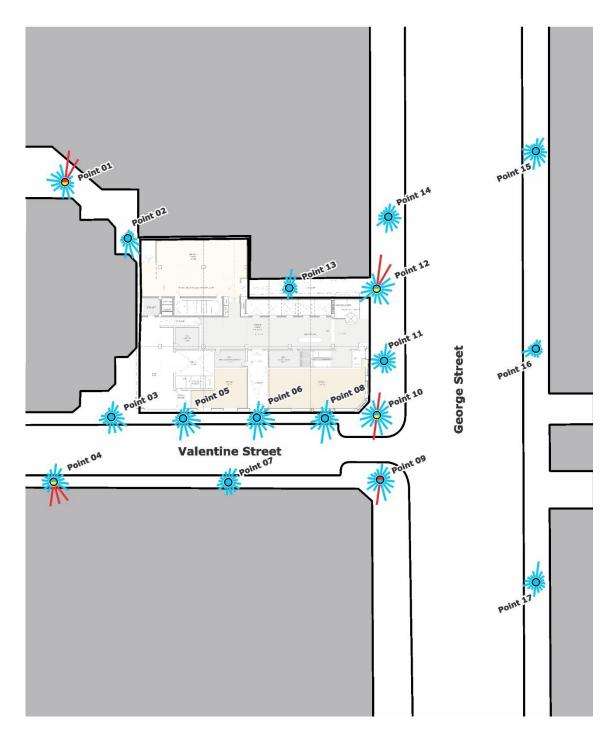


Figure 6c: Wind Tunnel Results – Alternative Envelope A Ground Level (results shown without treatments applied)

Wind Speed Magnitude from Directions Exceeding Criteria

- Wind Speed Magnitude from Directions Satisfying Criteria
- Passing Safety Limit and Comfort Criteria
- 🔵 🛛 Failing Safety Limit

- Failing Comfort Criteria
 - Failing Safety Limit and Comfort Criteria

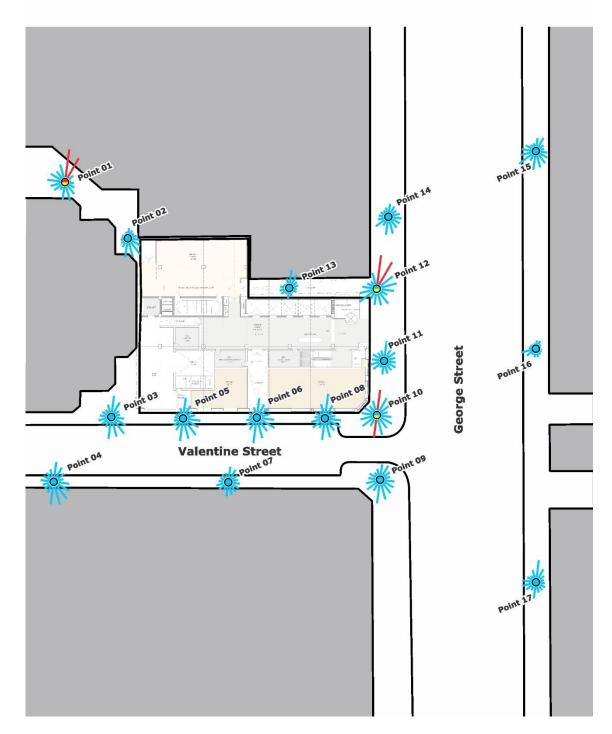


Figure 6d: Wind Tunnel Results – Alternative Envelope B Ground Level (results shown without treatments applied)

Wind Speed Magnitude from Directions Exceeding Criteria

- Wind Speed Magnitude from Directions Satisfying Criteria
- Passing Safety Limit and Comfort Criteria
- 🔵 🛛 Failing Safety Limit
- Failing Comfort Criteria
- Failing Safety Limit and Comfort Criteria

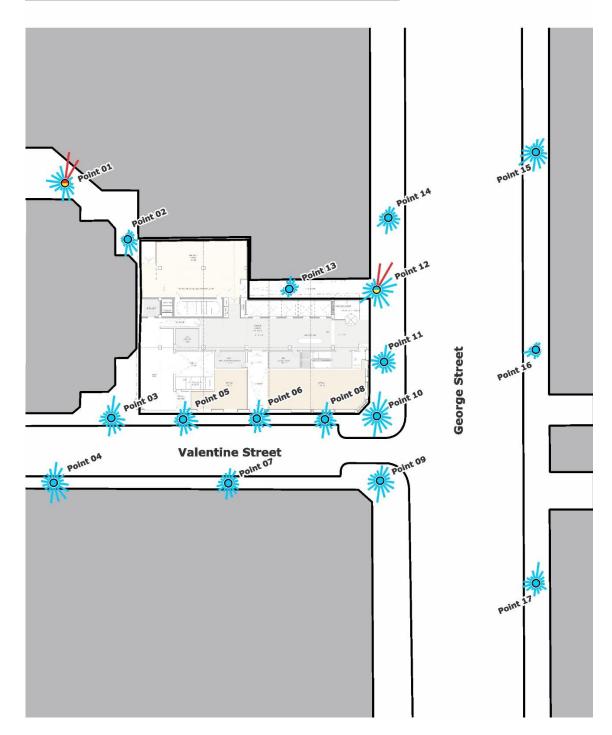
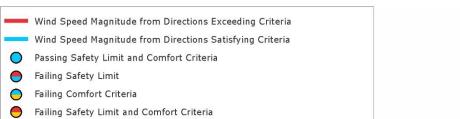


Figure 6e: Wind Tunnel Results – Alternative Envelope C Ground Level (results shown without treatments applied)





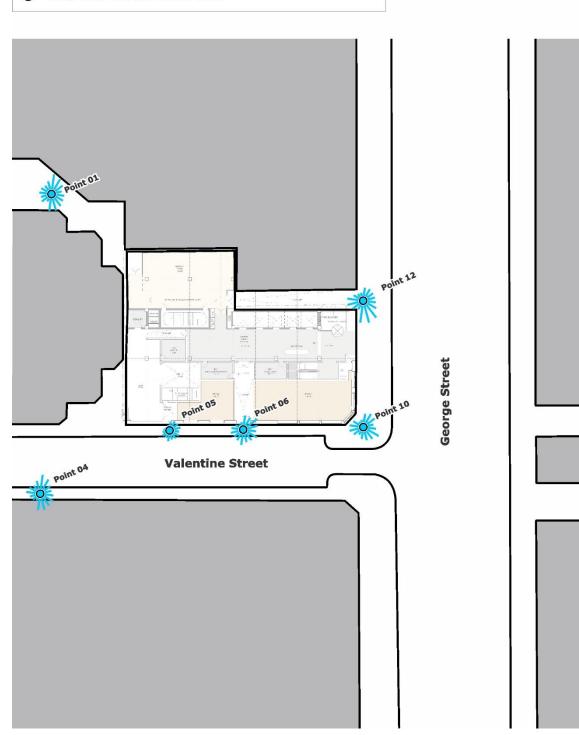


Figure 6f: Wind Tunnel Results – Existing Site Ground Level (results shown without treatments applied)

Study Point	(5% e	GEM exceedan	Annual Gust			:	Final Result
Study Point	Criterion (m/s)	Results (%)	Grade	Criterion (m/s)	Results (m/s)	Grade	rinai Kesult
Point 01	- 8.0	4%	Pass	24	23	Pass	Pass
Existing	0.0	1%	Pass	24	18	Pass	Pass
Point 02	8.0	0%	Pass	24	13	Pass	Pass
Point 03	8.0	0%	Pass	24	15	Pass	Pass
Point 04	0.0	2%	Pass	24	18	Pass	Pass
Existing	- 8.0	1%	Pass	24	17	Pass	Pass
Point 05	0.0	0%	Pass	24	13	Pass	Pass
Existing	- 8.0	0%	Pass	24	11	Pass	Pass
Point 06	<u> </u>	2%	Pass	24	16	Pass	Pass
Existing	- 6.0	0%	Pass	24	12	Pass	Pass
Point 07	8.0	0%	Pass	24	15	Pass	Pass
Point 08	8.0	1%	Pass	24	19	Pass	Pass
Point 09	8.0	4%	Pass	24	17	Pass	Pass
Point 10		2%	Pass	24	20	Pass	Pass
Existing	- 8.0	1%	Pass	24	18	Pass	Pass
Point 11	8.0	1%	Pass	24	20	Pass	Pass
Point 12	6.0	15%	Fail	24	22	Pass	Fail
Existing	- 6.0	4%	Pass	24	17	Pass	Pass
Point 13	8.0	0%	Pass	24	15	Pass	Pass
Point 14	8.0	1%	Pass	24	16	Pass	Pass
Point 15	8.0	1%	Pass	24	20	Pass	Pass
Point 16	8.0	0%	Pass	24	13	Pass	Pass
Point 17	8.0	1%	Pass	24	19	Pass	Pass

Table 5a: Wind Tunnel Results Summary (Base Case Massing)

Notes:

For any study points listed with two rows of results data, the second row is for the .

existing site conditions. Test results are shown without any treatments applied. The base case massing scenario has been tested to establish a level of wind comfort performance that is to be compared against for the proposed massing envelopes. Proposed massing envelopes are also •

required to satisfy the pedestrian comfort and safety criteria. Treatment recommendations are not required for the base case. .

Study	GEM (5% exceedance)			Annual Gust		Final	Description of	
Point	Criterion (m/s)	Results (%)	Grade	Criterion (m/s)	Results (m/s)	Grade	Result	Treatment
Point 01	8.0	10%	Fail	24	27	Fail	Fail	Modify NW corner of base
Existing	8.0	1%	Pass	24	18	Pass	Pass	of tower at podium level.
Point 02	8.0	1%	Pass	24	16	Pass	Pass	
Point 03	8.0	4%	Pass	24	22	Pass	Pass	
Point 04		4%	Pass		19	Pass	Pass	
Existing	8.0	1%	Pass	24	17	Pass	Pass	
Point 05		3%	Pass		22	Pass	Pass	
Existing	8.0	0%	Pass	24	11	Pass	Pass	
Point 06		7%	Fail		20	Pass	Fail	Ground level awning along
Existing	6.0	0%	Pass	24	12	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 07	8.0	0%	Pass	24	15	Pass	Pass	
Point 08	8.0	5%	Pass	24	24	Pass	Pass	
Point 09	8.0	2%	Pass	24	20	Pass	Pass	
Point 10		7%	Fail		23	Pass	Fail	Ground level awning along
Existing	8.0	1%	Pass	24	18	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 11	8.0	1%	Pass	24	18	Pass	Pass	
Point 12		9%	Fail		19	Pass	Fail	Ground level awning along
Existing	6.0	4%	Pass	24	17	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 13	8.0	0%	Pass	24	12	Pass	Pass	
Point 14	8.0	0%	Pass	24	17	Pass	Pass	
Point 15	8.0	1%	Pass	24	21	Pass	Pass	
Point 16	8.0	0%	Pass	24	16	Pass	Pass	
Point 17	8.0	1%	Pass	24	20	Pass	Pass	

Table 5b: Wind Tunnel Results Summary (Proposed DCP Envelope)

Notes:

• For any study points listed with two rows of results data, the second row is for the existing site conditions.

• Test results are shown without any treatments applied. If treatments are required, they are described in the Table above.

Study	(5% €	GEM exceedan	ce)	An	nual Gust	Gust		Description of
Point	Criterion (m/s)	Results (%)	Grade	Criterion (m/s)	Results (m/s)	Grade	Result	Treatment
Point 01	0.0	11%	Fail	24	29	Fail	Fail	Modify NW corner of base
Existing	- 8.0	1%	Pass	24	18	Pass	Pass	of tower at podium level.
Point 02	8.0	2%	Pass	24	17	Pass	Pass	
Point 03	8.0	1%	Pass	24	17	Pass	Pass	
Point 04		6%	Fail	2.4	21	Pass	Fail	
Existing	8.0	1%	Pass	24	17	Pass	Pass	
Point 05		2%	Pass	2.4	22	Pass	Pass	
Existing	8.0	0%	Pass	24	11	Pass	Pass	
Point 06		3%	Pass		17	Pass	Pass	
Existing	6.0	0%	Pass	24	12	Pass	Pass	
Point 07	8.0	0%	Pass	24	14	Pass	Pass	
Point 08	8.0	4%	Pass	24	23	Pass	Pass	
Point 09	8.0	4%	Pass	24	25	Fail	Fail	
Point 10		6%	Fail		24	Pass	Fail	Ground level awning along
Existing	8.0	1%	Pass	24	18	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 11	8.0	0%	Pass	24	16	Pass	Pass	
Point 12		14%	Fail		24	Pass	Fail	Ground level awning along
Existing	6.0	4%	Pass	24	17	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 13	8.0	0%	Pass	24	15	Pass	Pass	
Point 14	8.0	0%	Pass	24	16	Pass	Pass	
Point 15	8.0	1%	Pass	24	18	Pass	Pass	
Point 16	8.0	0%	Pass	24	15	Pass	Pass	
Point 17	8.0	1%	Pass	24	19	Pass	Pass	

Table 5c: Wind Tunnel Results Summary (Alternative Envelope A)

Notes:

.

For any study points listed with two rows of results data, the second row is for the existing site conditions. Test results are shown without any treatments applied. If treatments are required, they are described in the Table above.

Study	(5% €	GEM % exceedance) Annual Gust		:	Final	Description of		
Point	Criterion (m/s)	Results (%)	Grade	Criterion (m/s)	Results (m/s)	Grade	Result	Treatment
Point 01	- 8.0	12%	Fail	24	31	Fail	Fail	Modify NW corner of base
Existing	0.0	1%	Pass	24	18	Pass	Pass	of tower at podium level.
Point 02	8.0	2%	Pass	24	17	Pass	Pass	
Point 03	8.0	4%	Pass	24	22	Pass	Pass	
Point 04		5%	Pass	2.4	20	Pass	Pass	
Existing	8.0	1%	Pass	24	17	Pass	Pass	
Point 05		4%	Pass	2.4	23	Pass	Pass	
Existing	8.0	0%	Pass	24	11	Pass	Pass	
Point 06		5%	Pass		19	Pass	Pass	
Existing	6.0	0%	Pass	24	12	Pass	Pass	
Point 07	8.0	0%	Pass	24	14	Pass	Pass	
Point 08	8.0	4%	Pass	24	23	Pass	Pass	
Point 09	8.0	3%	Pass	24	24	Pass	Pass	
Point 10		6%	Fail		23	Pass	Fail	Ground level awning along
Existing	8.0	1%	Pass	24	18	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 11	8.0	1%	Pass	24	18	Pass	Pass	
Point 12		15%	Fail		24	Pass	Fail	Ground level awning along
Existing	6.0	4%	Pass	24	17	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 13	8.0	0%	Pass	24	16	Pass	Pass	
Point 14	8.0	1%	Pass	24	18	Pass	Pass	
Point 15	8.0	2%	Pass	24	22	Pass	Pass	
Point 16	8.0	0%	Pass	24	15	Pass	Pass	
Point 17	8.0	1%	Pass	24	20	Pass	Pass	

Table 5d: Wind Tunnel Results Summary (Alternative Envelope B)

Notes:

.

For any study points listed with two rows of results data, the second row is for the existing site conditions. Test results are shown without any treatments applied. If treatments are required, they are described in the Table above.

Study	(5% e	GEM exceedan	ce)	Annual Gust		Final	Description of	
Point	Criterion (m/s)	Results (%)	Grade	Criterion (m/s)	Results (m/s)	Grade	Result	Treatment
Point 01	- 8.0	11%	Fail	24	29	Fail	Fail	Modify NW corner of base
Existing	0.0	1%	Pass	24	18	Pass	Pass	of tower at podium level.
Point 02	8.0	0%	Pass	24	17	Pass	Pass	
Point 03	8.0	2%	Pass	24	21	Pass	Pass	
Point 04		3%	Pass	24	19	Pass	Pass	
Existing	- 8.0	1%	Pass	24	17	Pass	Pass	
Point 05		1%	Pass	24	17	Pass	Pass	
Existing	- 8.0	0%	Pass	24	11	Pass	Pass	
Point 06	6.0	1%	Pass	24	13	Pass	Pass	
Existing	- 6.0	0%	Pass	24	12	Pass	Pass	
Point 07	8.0	0%	Pass	24	16	Pass	Pass	
Point 08	8.0	1%	Pass	24	19	Pass	Pass	
Point 09	8.0	2%	Pass	24	22	Pass	Pass	
Point 10		4%	Pass	24	21	Pass	Pass	
Existing	- 8.0	1%	Pass	24	18	Pass	Pass	
Point 11	8.0	0%	Pass	24	15	Pass	Pass	
Point 12		13%	Fail		22	Pass	Fail	Ground level awning along
Existing	6.0	4%	Pass	24	17	Pass	Pass	the eastern and northern aspects. See Figure 7.
Point 13	8.0	0%	Pass	24	11	Pass	Pass	
Point 14	8.0	0%	Pass	24	16	Pass	Pass	
Point 15	8.0	2%	Pass	24	22	Pass	Pass	
Point 16	8.0	0%	Pass	24	13	Pass	Pass	
Point 17	8.0	1%	Pass	24	20	Pass	Pass	

Table 5e: Wind Tunnel Results Summary (Alternative Envelope C)

Notes:

For any study points listed with two rows of results data, the second row is for the existing site conditions.
 Test results are shown without any treatments applied. If treatments are required, they are described in the Table above.

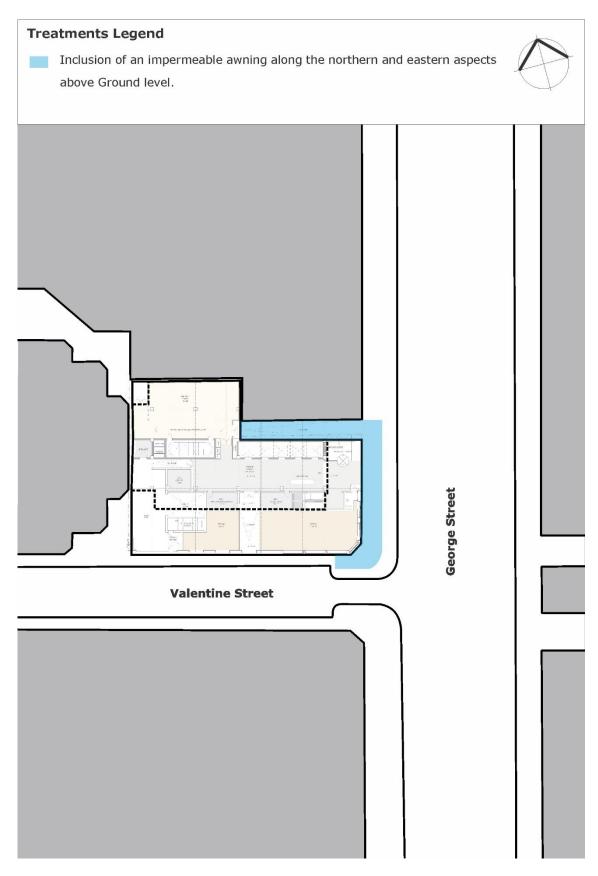


Figure 7: Suggested Treatments – Proposed DCP Envelope Ground Level

7 REFERENCES

American Society of Civil Engineers (ASCE), 2003, "Outdoor Human Comfort and its Assessment – State of the Art".

American Society of Civil Engineers (ASCE), ASCE-7-16, 2016, "Minimum Design Loads for Buildings and Other Structures".

Australasian Wind Engineering Society, QAM-1, 2019, "Quality Assurance Manual: Wind Engineering Studies of Buildings", edited by Rofail A.W., *et al.*

Australasian Wind Engineering Society (AWES), 2014, "Guidelines for Pedestrian Wind Effects Criteria".

Council on Tall Buildings and Urban Habitat (CTBUH), 2013, "Wind tunnel testing of high-rise buildings", CTBUH Technical Guides.

Davenport, A.G., 1972, "An approach to human comfort criteria for environmental conditions". Colloquium on Building Climatology, Stockholm.

Deaves, D.M. and Harris, R.I., 1978, "A mathematical model of the structure of strong winds." Construction Industry and Research Association (U.K), Report 76.

Engineering Science Data Unit, 1982, London, ESDU82026, "Strong Winds in the Atmospheric Boundary Layer, Part 1: Hourly Mean Wind Speeds", with Amendments A to E (issued in 2002).

Melbourne, W.H., 1978, "Criteria for Environmental Wind Conditions". *Journal of Wind Engineering and Industrial Aerodynamics*, vol. 3, pp241-249.

Rofail, A.W., and Kwok, K.C.S., 1991, "A Reliability Study of Wind Tunnel Results of Cladding Pressures". Proceedings of the 8th International Conference on Wind Engineering, Canada.

Rofail, A.W., 2007, "Comparison of Wind Environment Criteria against Field Observations". 12th International Conference of Wind Engineering, Cairns, Australia.

Standards Australia and Standards New Zealand, AS/NZS 1170.2, 2011, "SAA Wind Loading Standard, Part 2: Wind Actions".

APPENDIX A PUBLISHED ENVIRONMENTAL CRITERIA

A.1 Wind Effects on People

The acceptability of wind in an area is dependent upon the use of the area. For example, people walking or window-shopping will tolerate higher wind speeds than those seated at an outdoor restaurant. Quantifying wind comfort has been the subject of much research and many researchers, such as A.G. Davenport, T.V. Lawson, W.H. Melbourne, and A.D. Penwarden, have published criteria for pedestrian comfort for pedestrians in outdoor spaces for various types of activities. This section discusses and compares the various published criteria.

A.1.1 A.D. Penwarden (1973) Criteria for Mean Wind Speeds

A.D. Penwarden (1973) developed a modified version of the Beaufort scale which describes the effects of various wind intensities on people. Table A.1 presents the modified Beaufort scale. Note that the effects listed in this table refers to wind conditions occurring frequently over the averaging time (a probability of occurrence exceeding 5%). Higher ranges of wind speeds can be tolerated for rarer events.

Type of Winds	Beaufort Number	Hourly Mean Wind Speed (m/s)	Effects
Calm	0	0 - 0.25	
Calm, light air	1	0 25 - 1.55	No noticeable wind
Light breeze	2	1.55 - 3.35	Wind felt on face
Gentle breeze	3	3.35 - 5.45	Hair is disturbed, clothing flaps, newspapers difficult to read
Moderate breeze	4	5.45 - 7.95	Raises dust, dry soil and loose paper, hair disarranged
Fresh breeze	5	7.95 - 10.75	Force of wind felt on body, danger of stumbling
Strong breeze	6	10.75 - 13.85	Umbrellas used with difficulty, hair blown straight, difficult to walk steadily, wind noise on ears unpleasant
Near gale	7	13.85 - 17.15	Inconvenience felt when walking
Gale	8	17.15 - 20.75	Generally impedes progress, difficulty balancing in gusts
Strong gale	9	20.75 - 24.45	People blown over

Table A.1: Summary of Wind Effects on People (A.D. Penwarden, 1973)

A.1.2 A.G. Davenport (1972) Criteria for Mean Wind Speeds

A.G. Davenport (1972) also determined a set of criteria in terms of the Beaufort scale and for various return periods. Table A.2 presents a summary of the criteria based on a probability of exceedance of 5%.

Classification	Activities	5% exceedance Mean Wind Speed (m/s)
Walking Fast	Acceptable for walking, main public accessways.	7.5 - 10.0
Strolling, Skating	Slow walking, etc.	5.5 - 7.5
Short Exposure Activities	Generally acceptable for walking & short duration stationary activities such as window-shopping, standing or sitting in plazas.	3.5 - 5.5
Long Exposure Activities	Generally acceptable for long duration stationary activities such as in outdoor restaurants & theatres and in parks.	0 - 3.5

Table A.2: Criteria by A.G. Davenport (1972)

A.1.3 T.V. Lawson (1975) Criteria for Mean Wind Speeds

In 1973, T.V. Lawson, while referring to the Beaufort wind speeds of A.D. Penwarden (1973) (as listed in Table A.1), quoted that a Beaufort 4 wind speed would be acceptable if it is not exceeded for more than 4% of the time, and that a Beaufort 6 wind speed would be unacceptable if it is exceeded more than 2% of the time. Later, in 1975, T.V. Lawson presented a set of criteria very similar to those presented in A.G. Davenport (1972) (as listed in Table A.2). These criteria are presented in Table A.3 and Table A.4 for safety and comfort respectively.

Classification	Activities	Annual Mean Wind Speed (m/s)
Safety (all weather areas)	Accessible by the general public.	0 - 15
Safety (fair weather areas)	Private areas, balconies/terraces, etc.	0 - 20

Table A.3: Safety Criteria by T.V. Lawson (1975)

Table A.4: Comfort Criteria by T.V. Lawson (1975)

Classification	Activities	5% exceedance Mean Wind Speed (m/s)
Business Walking	Objective Walking from A to B.	8 - 10
Pedestrian Walking	Slow walking, etc.	6 - 8
Short Exposure Activities	Pedestrian standing or sitting for short times.	4 - 6
Long Exposure Activities	Pedestrian sitting for a long duration.	0 - 4

A.1.4 W.H. Melbourne (1978) Criteria for Gust Wind Speeds

W.H. Melbourne (1978) introduced a set of criteria for the assessment of environmental wind conditions that were developed for a temperature range of 10°C to 30°C and for people suitably dressed for outdoor conditions. These criteria are presented in Table A.5, and are based on maximum gust wind speeds with a probability of exceedance of once per year.

Classification	Human Activities	Annual Gust Wind Speed (m/s)
Limit for Safety	Completely unacceptable: people likely to get blown over.	23
Marginal	Unacceptable as main public accessways.	16 - 23
Comfortable Walking	Acceptable for walking, main public accessways	13 - 16
Short Exposure Activities	Generally acceptable for walking & short duration stationary activities such as window-shopping, standing or sitting in plazas.	10 - 13
Long Exposure Activities	Generally acceptable for long duration stationary activities such as in outdoor restaurants & theatres and in parks.	0 - 10

Table A.5: Criteria by W.H. Melbourne (1978)

A.2 Comparison of the Published Wind Speed Criteria

W.H. Melbourne (1978) presented a comparison of the criteria of various researchers on a probabilistic basis. Figure A.1 presents the results of this comparison, and indicates that the criteria of W.H. Melbourne (1978) are comparatively quite conservative. This conclusion was also observed by A.W. Rofail (2007) when undertaking on-site remedial studies. The results of A.W. Rofail (2007) concluded that the criteria by W.H. Melbourne (1978) generally overstates the wind effects in a typical urban setting due to the assumption of a fixed 15% turbulence intensity for all areas. It was observed in A.W. Rofail (2007) that the 15% turbulence intensity assumption is not real and that the turbulence intensities at 1.5m above ground is at least 20% and in a suburban or urban setting is generally in the range of 30% to 60%.

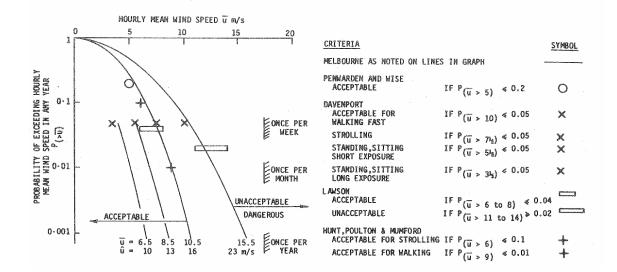


Figure A.1: Comparison of Various Mean and Gust Wind Environment Criteria, assuming 15% turbulence and a Gust Factor of 1.5 (W.H. Melbourne, 1978)

A.3 References relating to Pedestrian Comfort Criteria

Davenport, A.G., 1972, "An approach to human comfort criteria for environmental conditions". Colloquium on Building Climatology, Stockholm.

Davenport, A.G., 1977, "The prediction of risk under wind loading", 2nd International Conference on Structural Safety and Reliability, Munich, Germany, pp511-538.

Lawson, T.V., 1973, "The wind environment of buildings: a logical approach to the establishment of criteria". Bristol University, Department of Aeronautical Engineering.

Lawson, T.V., 1975, "The determination of the wind environment of a building complex before construction". Bristol University, Department of Aeronautical Engineering.

Melbourne, W.H., 1978, "Criteria for Environmental Wind Conditions". Journal of Wind Engineering and Industrial Aerodynamics, vol. 3, pp241-249.

Penwarden, A.D. (1973). "Acceptable Wind Speeds in Towns", Building Science, vol. 8: pp259–267

Penwarden, A.D., Wise A.F.E., 1975, "Wind Environment Around Buildings". Building Research Establishment Report, London.

Rofail, A.W., 2007, "Comparison of Wind Environment Criteria against Field Observations". 12th International Conference of Wind Engineering, Cairns, Australia.

APPENDIX B DATA ACQUISITION

The wind tunnel testing procedures for this study were based on the guidelines set out in the Australasian Wind Engineering Society Quality Assurance Manual (AWES-QAM-1-2019), ASCE 7-16 (Chapter C31), and CTBUH (2013).

The wind speed measurements for the wind tunnel study were acquired as coefficients by Dantec hot-wire anemometers and converted to full-scale wind speeds using details of the regional wind climate obtained from an analysis of directional wind speed recordings from the local meteorological recording station(s).

B.1 Measurement of the Velocity Coefficients

The study model and proximity model were setup within the wind tunnel which was configured to the appropriate boundary layer profile, and the wind velocity measurements were monitored using Dantec hot-wire probe anemometers at selected critical outdoor locations. The anemometers were positioned at each study location at a full-scale height of approximately 1.5m above ground/slab level. The support of the probe was mounted such that the probe wire was vertical as much as possible to ensure that the measured wind speeds are independent of wind direction along the horizontal plane. In addition, care was taken in the alignment of the probe wire and in avoiding wall-heating effects.

Wind speed measurements were made in the wind tunnel for 16 wind directions, at 22.5° increments. The output from the hot-wire probes was obtained using a National Instruments 12-bit data acquisition card. The data was acquired for each wind direction using a sample rate of 1024Hz. The sample length was determined to produce a full-scale sample time that is sufficient for this type of study.

The mean, gust and standard deviation velocity coefficients were measured in the wind tunnel. The gust velocity coefficients were also derived for each wind direction from by the following relation:

$$\hat{C}_V = \bar{C}_V + g \cdot \sigma_{C_V}$$

Where:

 \hat{C}_V is the gust coefficient.

- \bar{C}_V is the mean coefficient.
- $g_{\rm }$ $\,$ is the peak factor, taken as 3.0 for a 3s gust and 3.4 for a 0.5s gust.
- σ_{C_V} is the standard deviation of coefficient measurement.

B.1

B.2 Calculation of the Full-Scale Results

The full-scale results determine if the wind conditions at a study location satisfy the designated criteria of that location. More specifically, the full-scale results need to determine the probability of exceedance of a given wind speed at a study location. To determine the probability of exceedance, the measured velocity coefficients were combined with a statistical model of the local wind climate that relates wind speed to a probability of exceedance. Details of the wind climate model are outlined in Section 4 of the main report.

The statistical model of the wind climate includes the impact of wind directionality as any local variations in wind speed or frequency with wind direction. This is important as the wind directions that produce the highest wind speed events for a region may not coincide with the most wind exposed direction at the site.

The methodology adopted for the derivation of the full-scale results for the maximum gust and the GEM wind speeds are outlined in the following sub-sections.

B.2.1 Maximum Gust Wind Speeds

The full-scale maximum gust wind speed at each study point location is derived from the measured coefficient using the following relationship:

$$V_{study} = V_{ref,RH} \left(\frac{k_{200m,tr,T=1hr}}{k_{RH,tr,T=1hr}} \right) C_V$$
B.2

Where:

 V_{study} is the full-scale wind speed at the study point location, in m/s.

- $V_{ref,RH}$ is the full-scale reference wind speed, measured 3m upstream at the study reference height. This value is determined by combining the directional wind speed data for the region (detailed in Section 4) and the upwind terrain and height multipliers for the site (detailed in Section 3).
- $k_{200m,tr,T=1hr}$ is the standard deviation of the wind speed.
 - $k_{RH,tr,T=1hr}$ is the hourly mean terrain and height multiplier at the study reference height (see Section 3).
 - C_V is the velocity coefficient measurement obtained from the hot-wire anemometer, which is derived from the following relationship:

$$C_V = \frac{C_{V,study}}{C_{V,200m}}$$

B.3

Where:

- $C_{V,study}$ is the coefficient measurement obtained from the hot-wire anemometer at the study point location.
- $C_{V,200m}$ is the coefficient measurement obtained from the hot-wire anemometer at the free-stream reference location at 200m height upwind of the model in the wind tunnel.

The value of $V_{ref,RH}$ varies with each prevailing wind direction. Wind directions where there is a high probability that a strong wind will occur have a higher directional wind speed than other directions. To determine the directional wind speeds, a probability level must be assigned for each wind direction. These probability levels are set following the approach used in AS/NZS1170.2:2011, which assumes that the major contributions to the combined probability of exceedance of a typical load effect comes from only two 45 degree sectors.

B.2.2 Maximum Gust-Equivalent Mean Wind Speeds

The contribution to the probability of exceedance of a specified wind speed (ie: the desired wind speed for pedestrian comfort, as per the criteria) was calculated for each wind direction. These contributions are then combined over all wind directions to calculate the total probability of exceedance of the specified wind speed. To calculate the probability of exceedance for a specified wind speed a statistical wind climate model was used to describe the relationship between directional wind speeds and the probability of exceedance. A detailed description of the methodology is given by T.V. Lawson (1980).

The criteria used in this study is referenced to a probability of exceedance of 5% of a specified wind speed.

B.3 References relating to Data Acquisition

American Society of Civil Engineers (ASCE), ASCE-7-16, 2016, "Minimum Design Loads for Buildings and Other Structures".

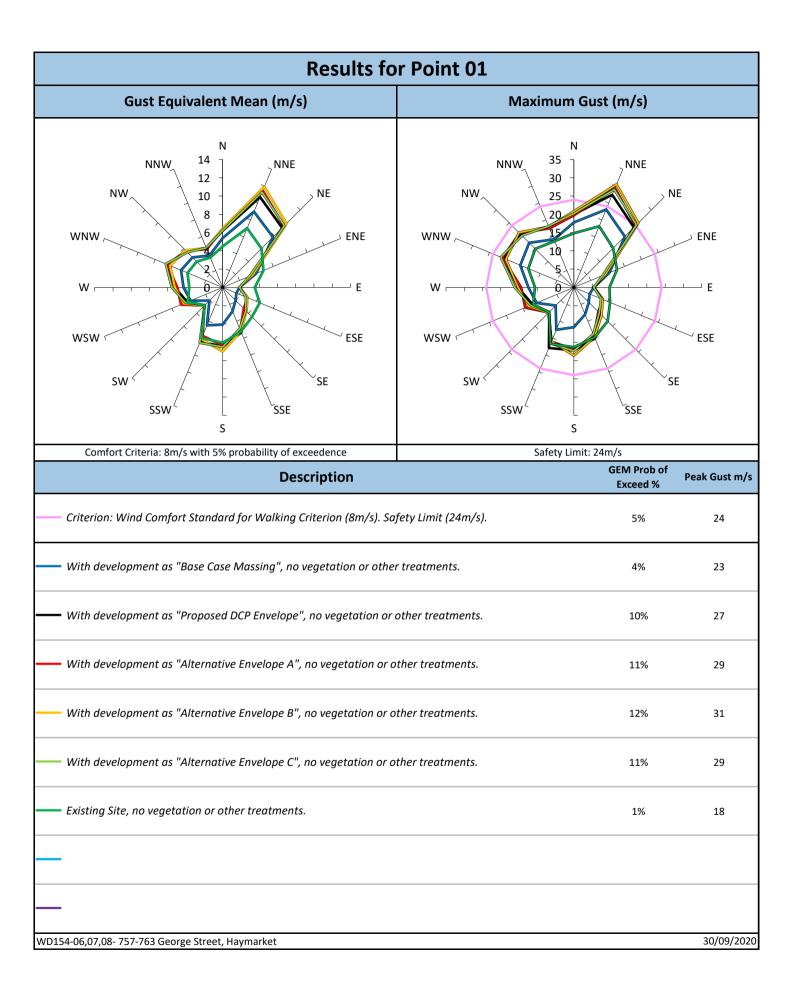
Australasian Wind Engineering Society, QAM-1, 2019, "Quality Assurance Manual: Wind Engineering Studies of Buildings", edited by Rofail A.W., *et al.*

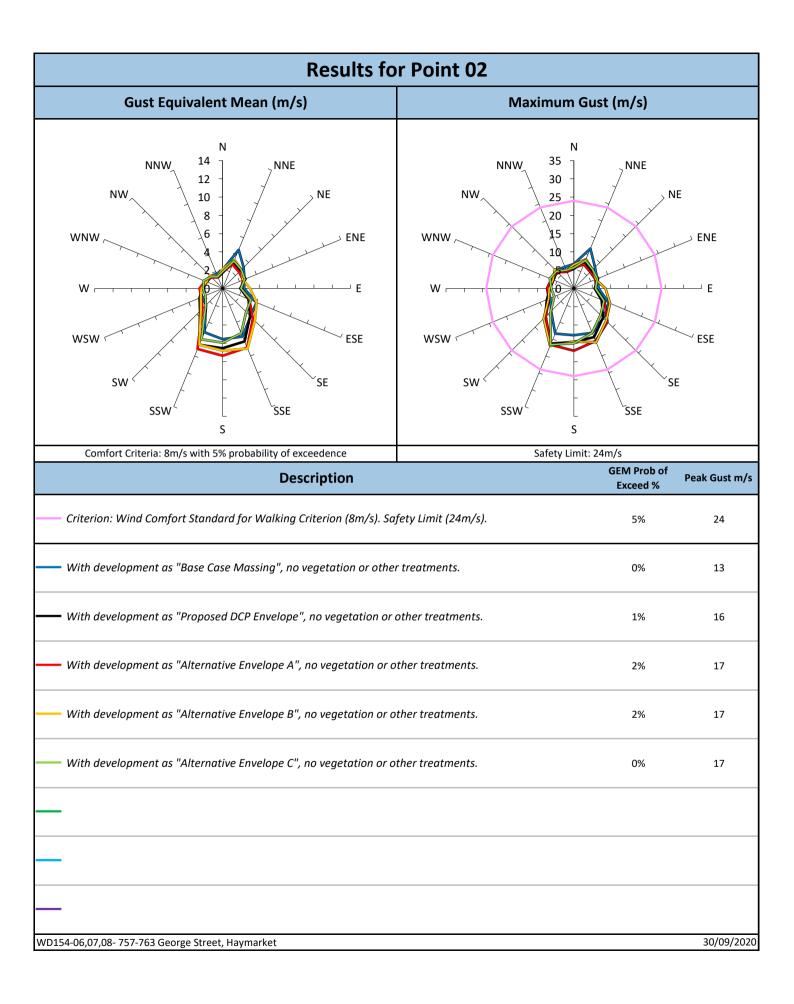
Council on Tall Buildings and Urban Habitat (CTBUH), 2013, "Wind tunnel testing of high-rise buildings", CTBUH Technical Guides.

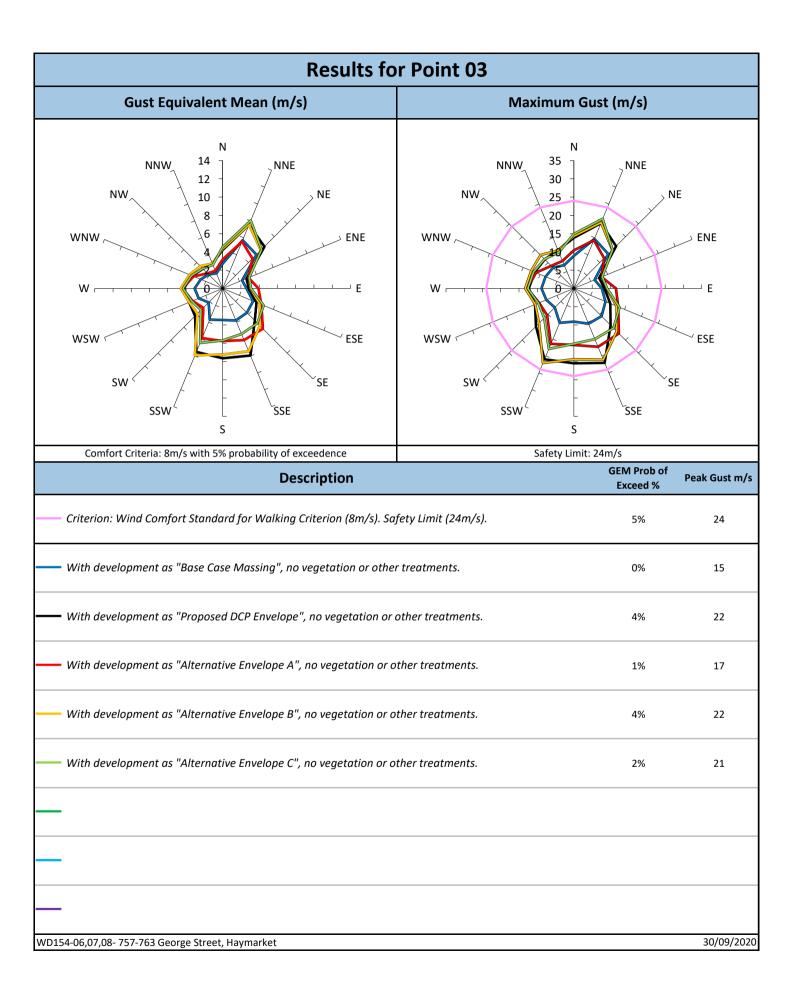
Lawson, T.V., 1980, "Wind Effects on Buildings - Volume 1, Design Applications". Applied Science Publishers Ltd, Ripple Road, Barking, Essex, England.

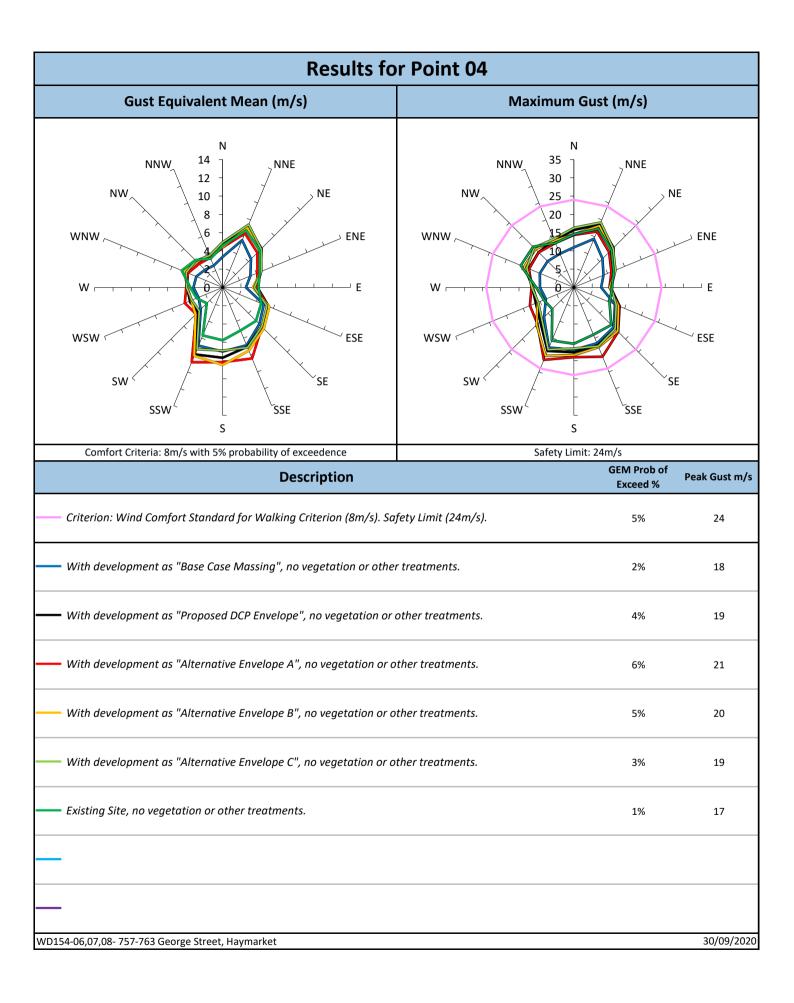
Standards Australia and Standards New Zealand, AS/NZS 1170.2, 2011, "SAA Wind Loading Standard, Part 2: Wind Actions".

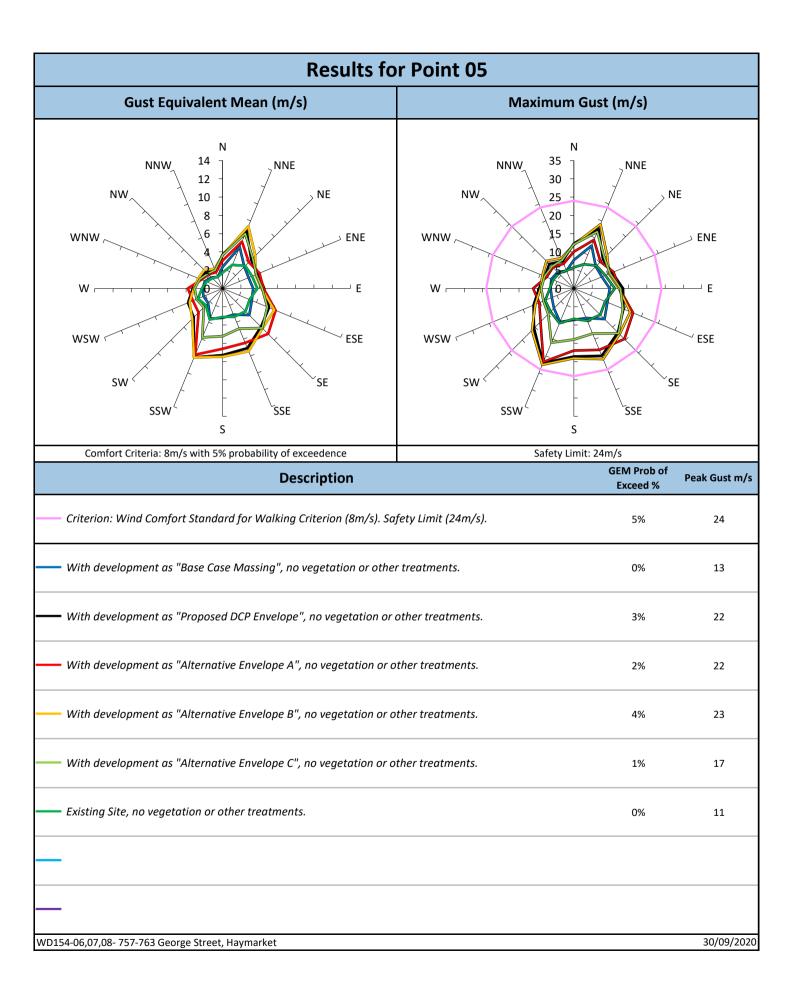
APPENDIX C DIRECTIONAL PLOTS OF WIND TUNNEL RESULTS

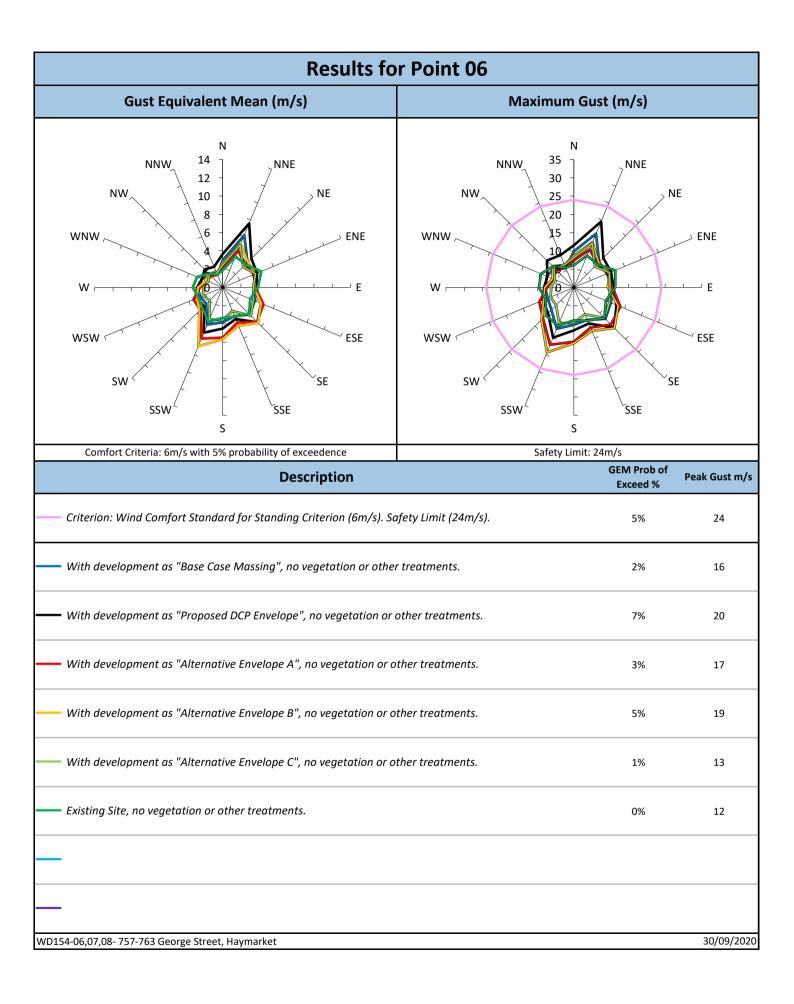


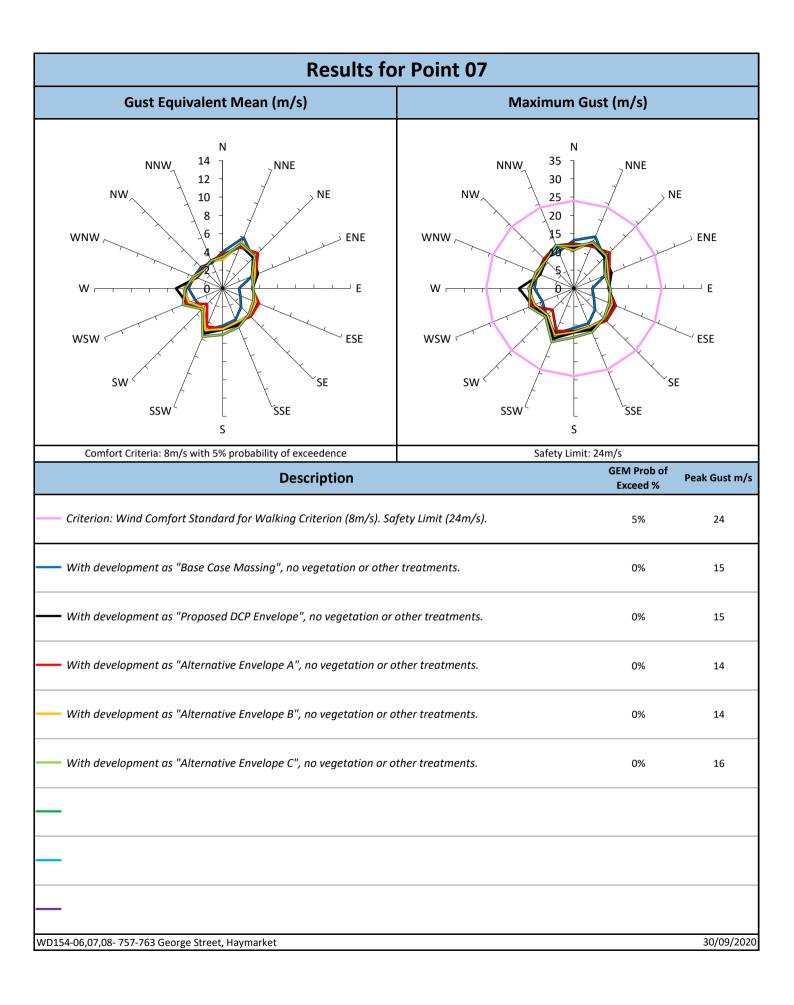


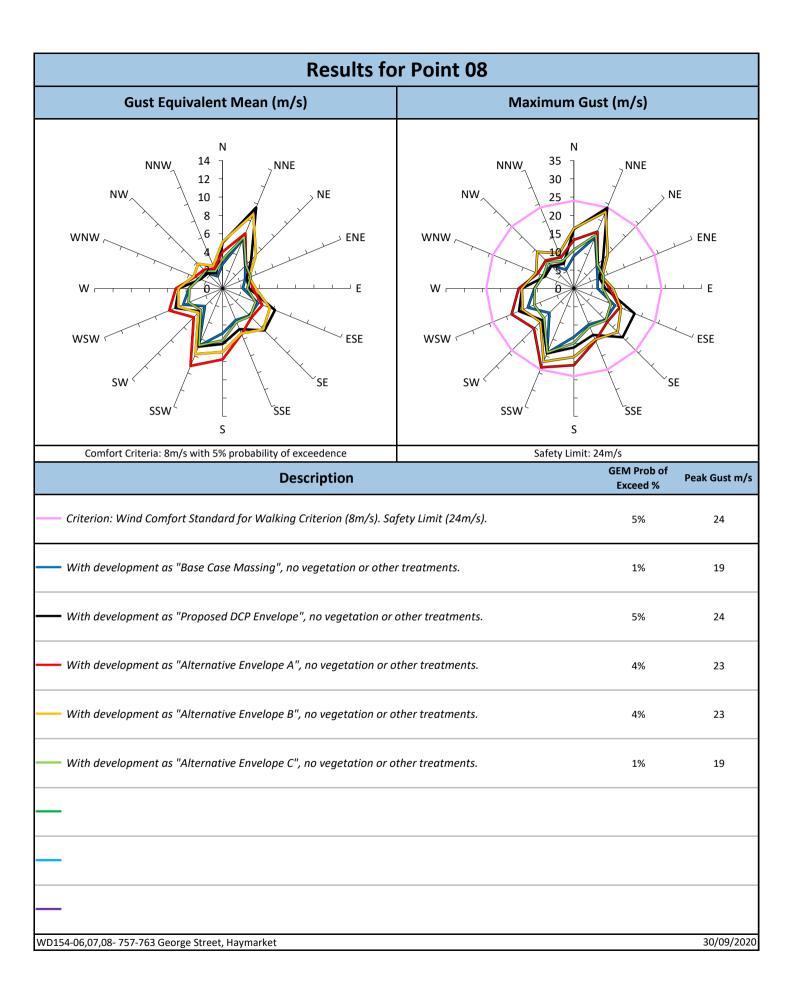


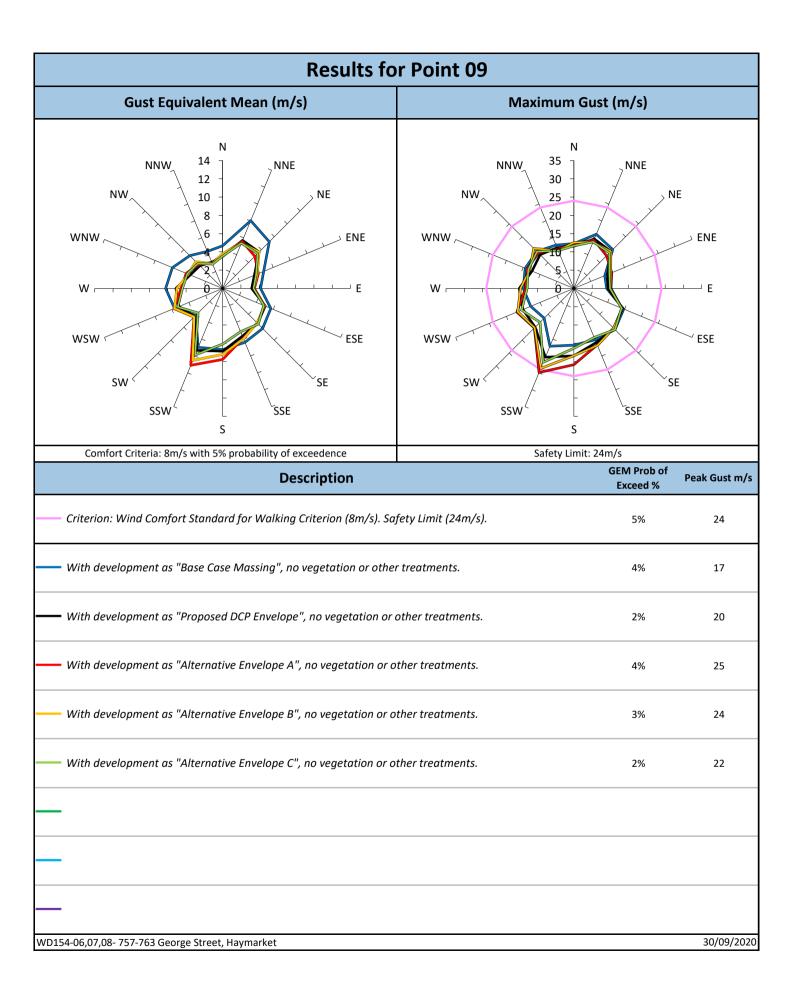


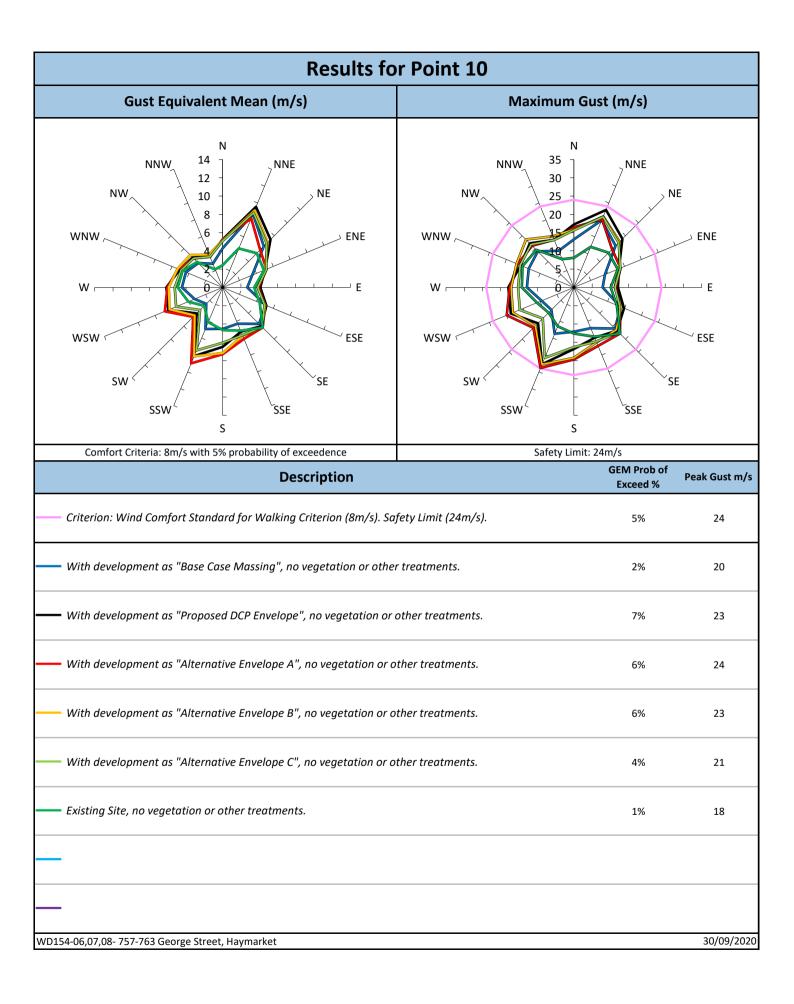


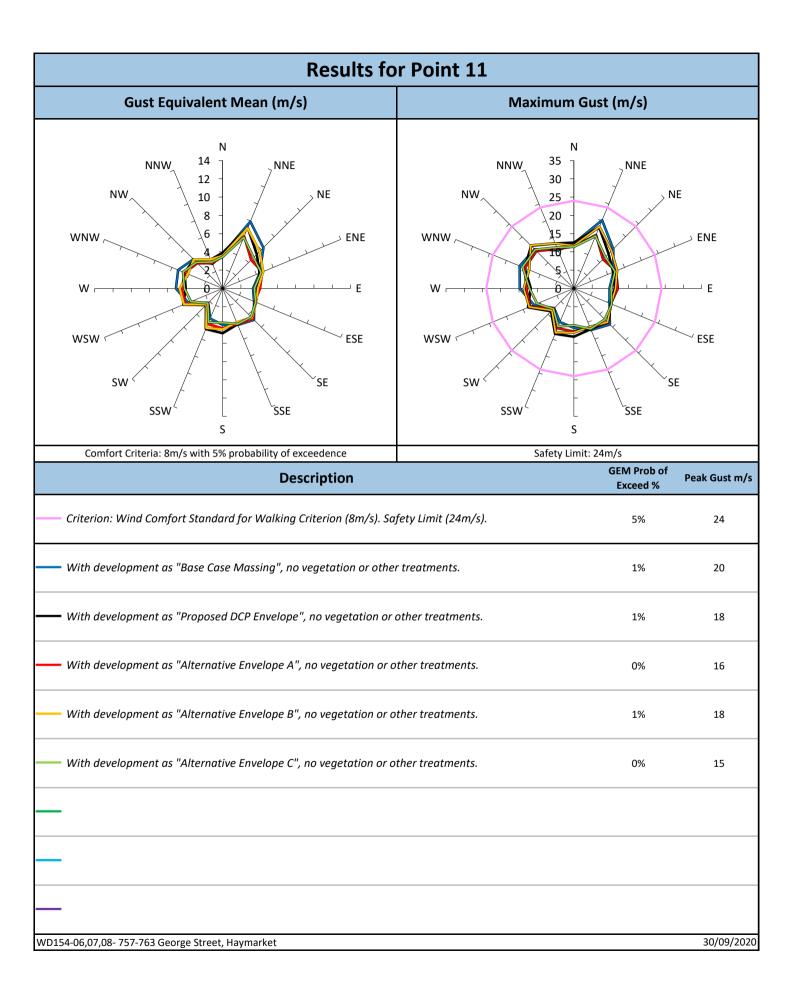


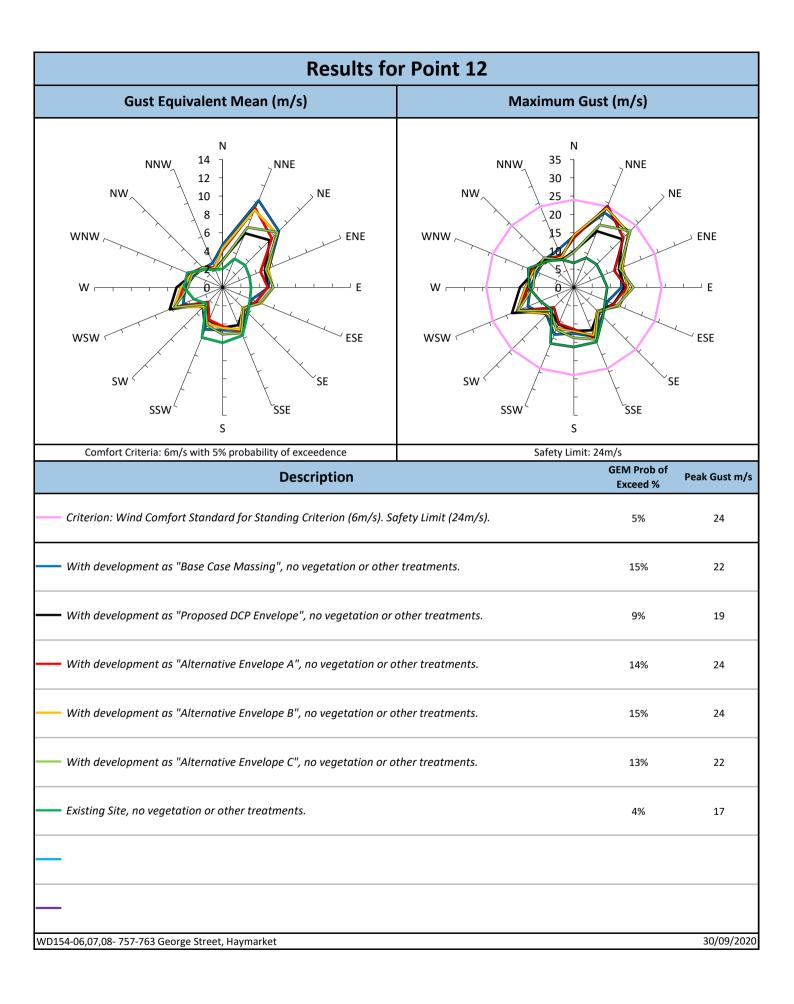


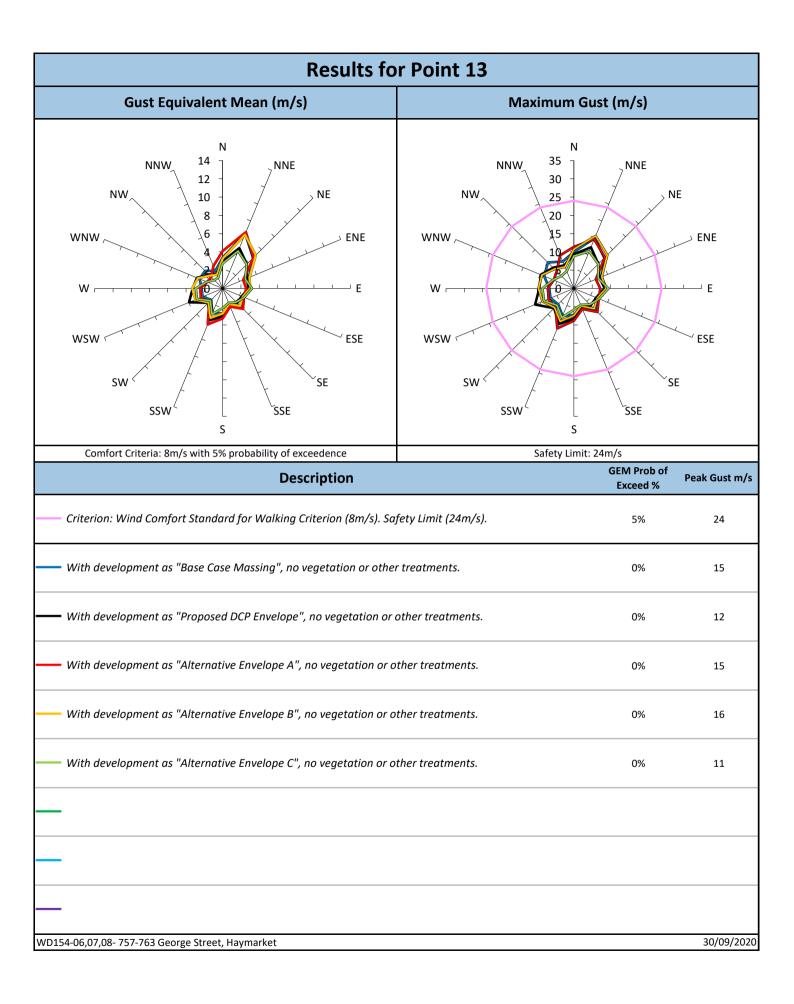


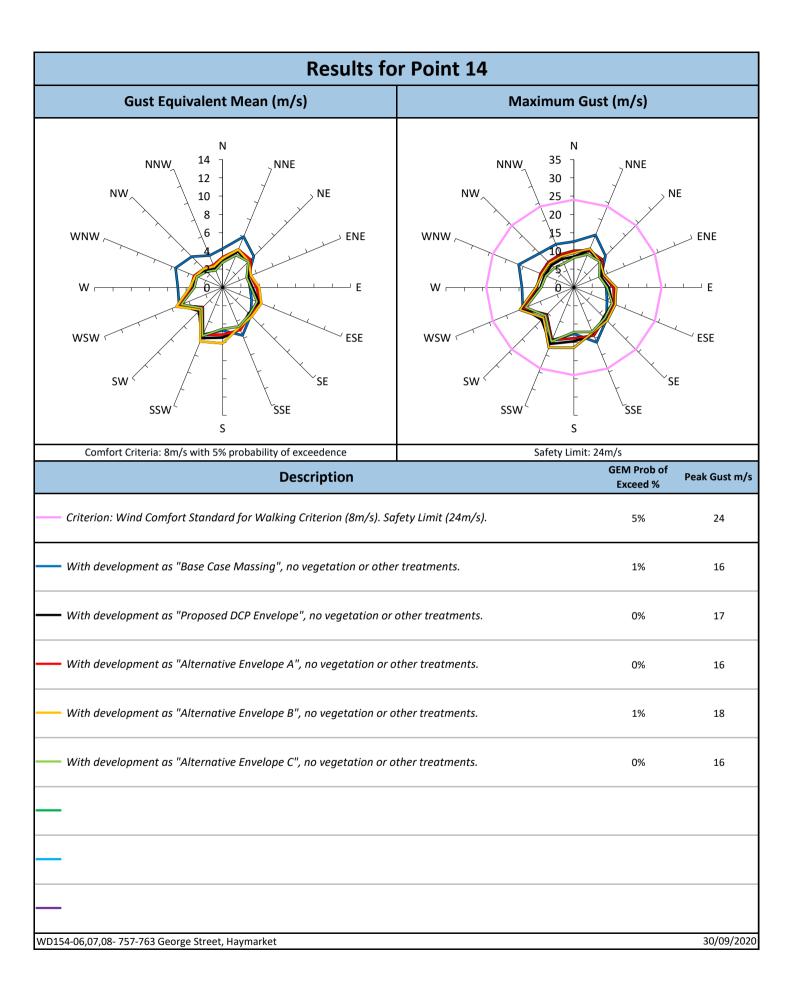


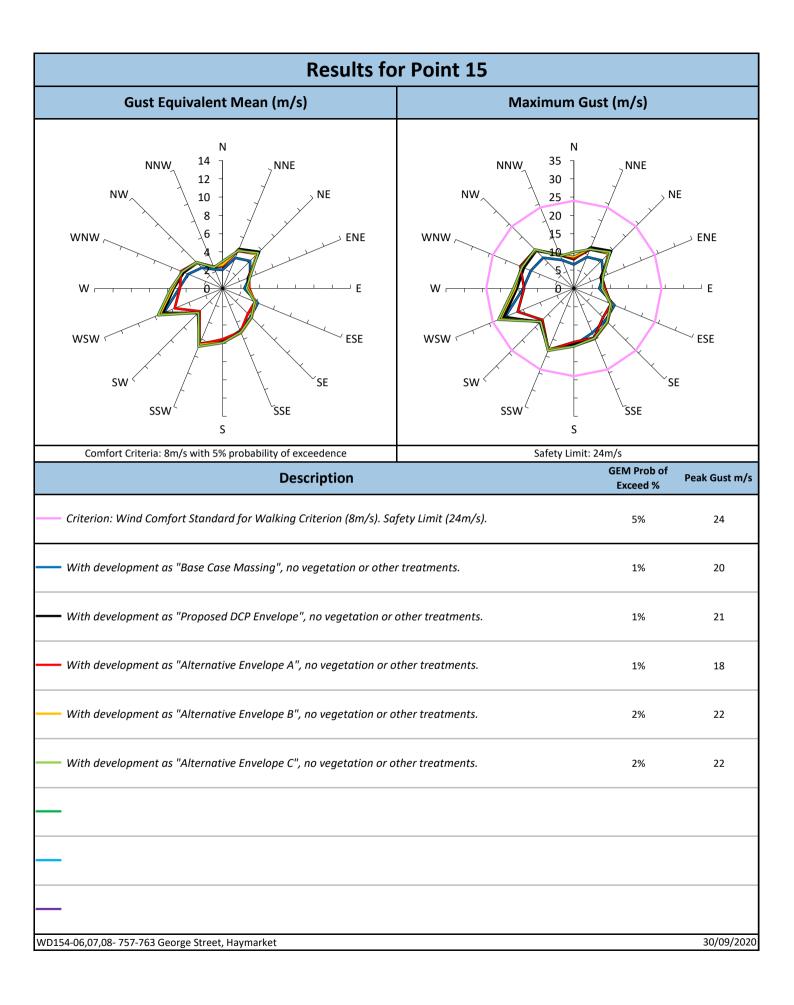


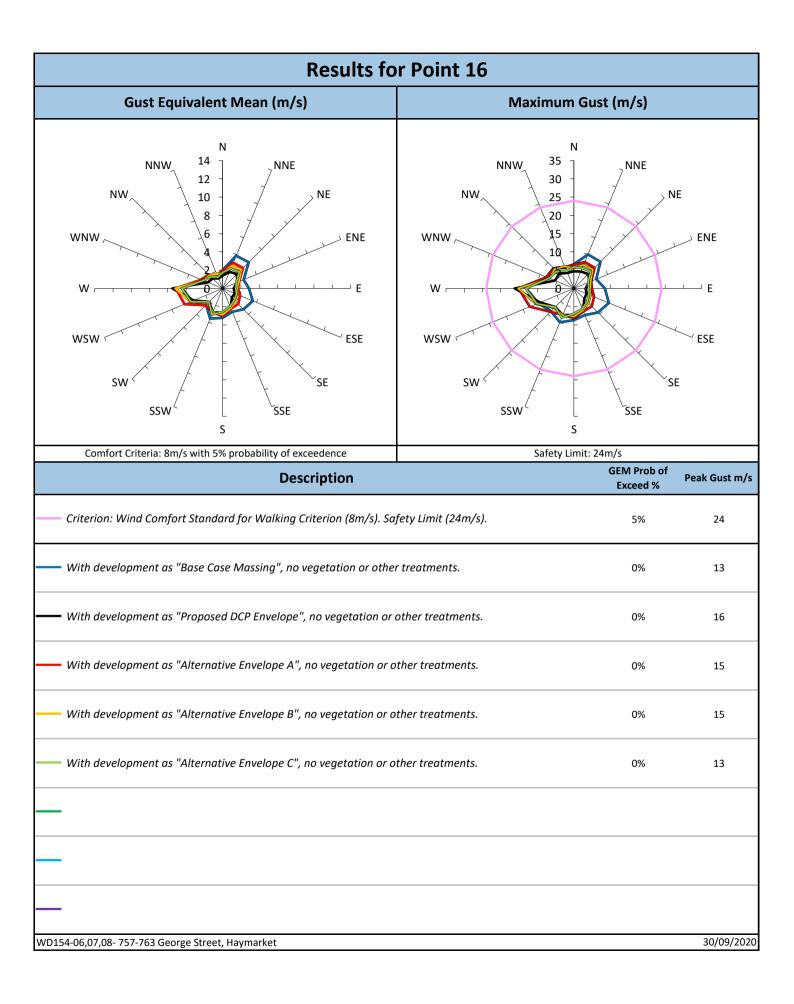


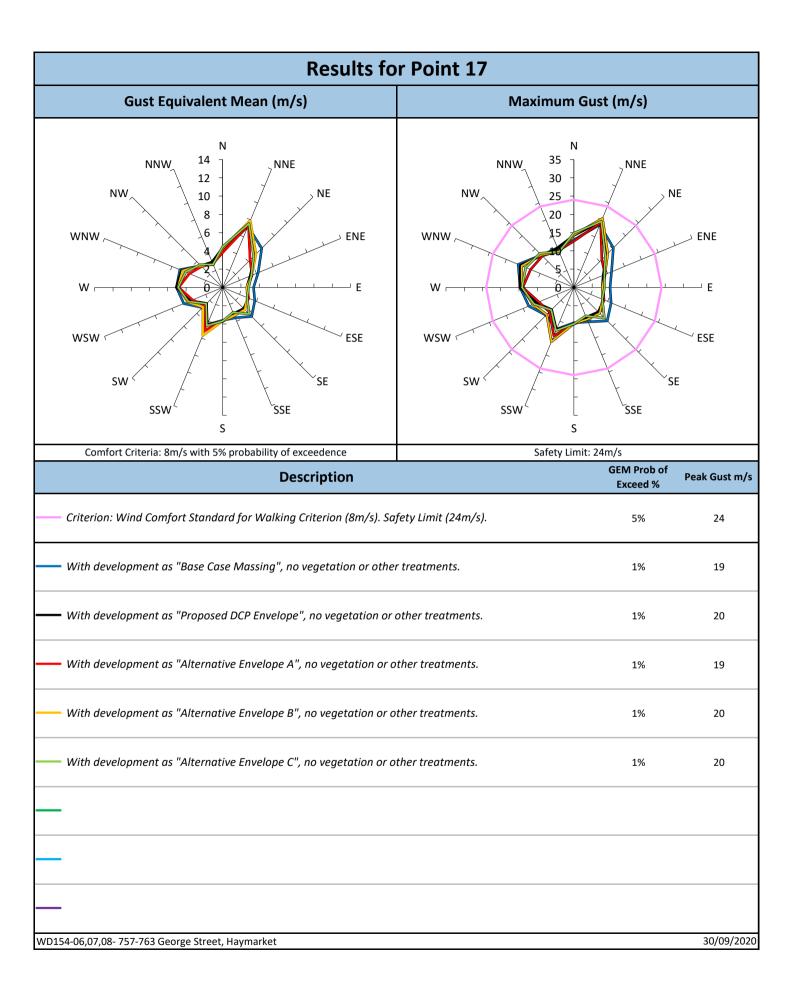




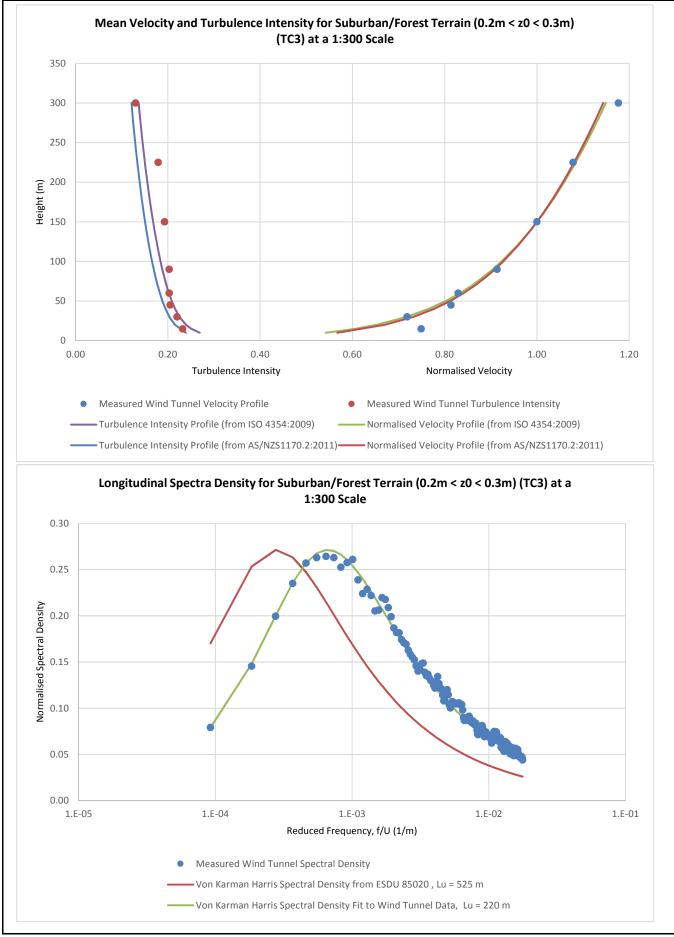








APPENDIX D VELOCITY AND TURBULENCE INTENSITY PROFILES



Windtech Consultants